



# Eklutna Fish & Wildlife Program

## Eklutna Dam Spillway Modifications

### Design Documentation Report

15% Design  
Revision No. 0



April 2024

PLACEHOLDER  
FOR  
ENGINEER  
STAMP

Date:

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## Revision Log

Revision No.	Date	Revision Description
0	April 18, 2024	15% Design – Initial Draft
1	April 25, 2024	15% Design – Final Draft

## **1.0 Introduction**

### **1.1 Purpose**

The purpose of this report is to present the design documentation associated with the development of the Spillway Modifications as part of the Eklutna Fish & Wildlife Program (Project). The design documentation report (DDR) summarizes the pertinent data, design criteria, and standards used as the basis for the 15% level of design of the Project. The DDR documents the data, criteria, and standards by engineering discipline, and provides information about the operation of the design components associated with the Project.

The information presented within this DDR will be updated throughout the design development process to reflect refinement of the Project components and facilities, and advancement of the design analysis and details.

### **1.2 Location**

The Project site is located in Southcentral Alaska approximately 30 miles northeast of downtown Anchorage near the Native Village of Eklutna. The Project footprint spans lands owned and managed by the State of Alaska, Eklutna Inc., and the Bureau of Land Management, with the dam and proposed modification to the dam spillway occurring on land owned by the State of Alaska. As proposed, the Project involves modifying the spillway within Eklutna Dam by removing the uncontrolled ogee crest overflow structure and replacing it with a fixed wheel gate and hoist for allowing controlled spills from Eklutna Lake. The location of Eklutna Dam and its vicinity to other features of the overall project is presented in Figure 1-1.

### **1.3 Background and Objectives**

As part of the Eklutna Fish & Wildlife Program, Chugach Electric Association, Matanuska Electric Association, and the Municipality of Anchorage, known collectively as the Owners of the Eklutna Hydroelectric Project (Owners) are studying various means of providing water to the Eklutna River to mitigate damages to, and enhance fish and wildlife impacted by development and continuing operation of the Project. During the consultation process with various stakeholders to the Project, requests were made to replace the existing overflow spillway at the dam with a fixed wheel gate because either (1) climate change may cause inflows to the reservoir to increase significantly, which may increase the likelihood of future spill events, and a fixed wheel gate will allow the Project Owners to better manage those future spill events, or (2) while modeling results show that the default channel maintenance flow regime will maintain spawning gravels in the wetted reach of the Eklutna River, future

monitoring may show that a higher magnitude channel maintenance flow that exceeds the combined hydraulic capacity of the existing outlet gate and the Eklutna River Release Facility may be warranted. The purpose of this report is to document the design criteria and analyses performed for the engineering development of the spillway modifications to Eklutna Dam.

## 1.4 Report Organization

This DDR is a record of the design effort for the Project and specifically describes the details of the design process and work effort for the Dam Outlet Modifications. The DDR consists of a summary of the design elements, criteria, methods and approach, engineering calculations, and pertinent references. The major report sections and intended purpose are presented in Table 1-1.

**Table 1-1. Major Report Sections and Purpose.**

Section	Description	Purpose
1	Introduction and Background	Presents the Project background, purpose, location, authorization, objectives, feature scope, and the report organization.
2	General Description	Summarizes the basic design criteria that are used as the basis of design.
3	Existing Infrastructure	Summarizes the existing infrastructure related to the Project.
4	Project Infrastructure and Operation	Describes the proposed modifications for the Eklutna Dam spillway.
5	Structural Design	Includes information related to the structural design of the fixed wheel gate, guides, and stability of the dam.
6	Mechanical Design	Includes information related to the mechanical design of the gate hoist.
7	Electrical and Instrumentation / Control Design	Includes information related to the electrical design and instrumentation and controls associated with the spillway modifications.
8	References	Documents the references used in developing the design.

Section	Description	Purpose
<b>Appendices</b>		
A	15% Design Drawings	Presents the 15% design drawings for the Eklutna Dam Spillway Modifications
B	Class 4 OPCC	Presents the Class 4 Opinion of Probable Construction Costs for the Eklutna Dam Spillway Modifications.

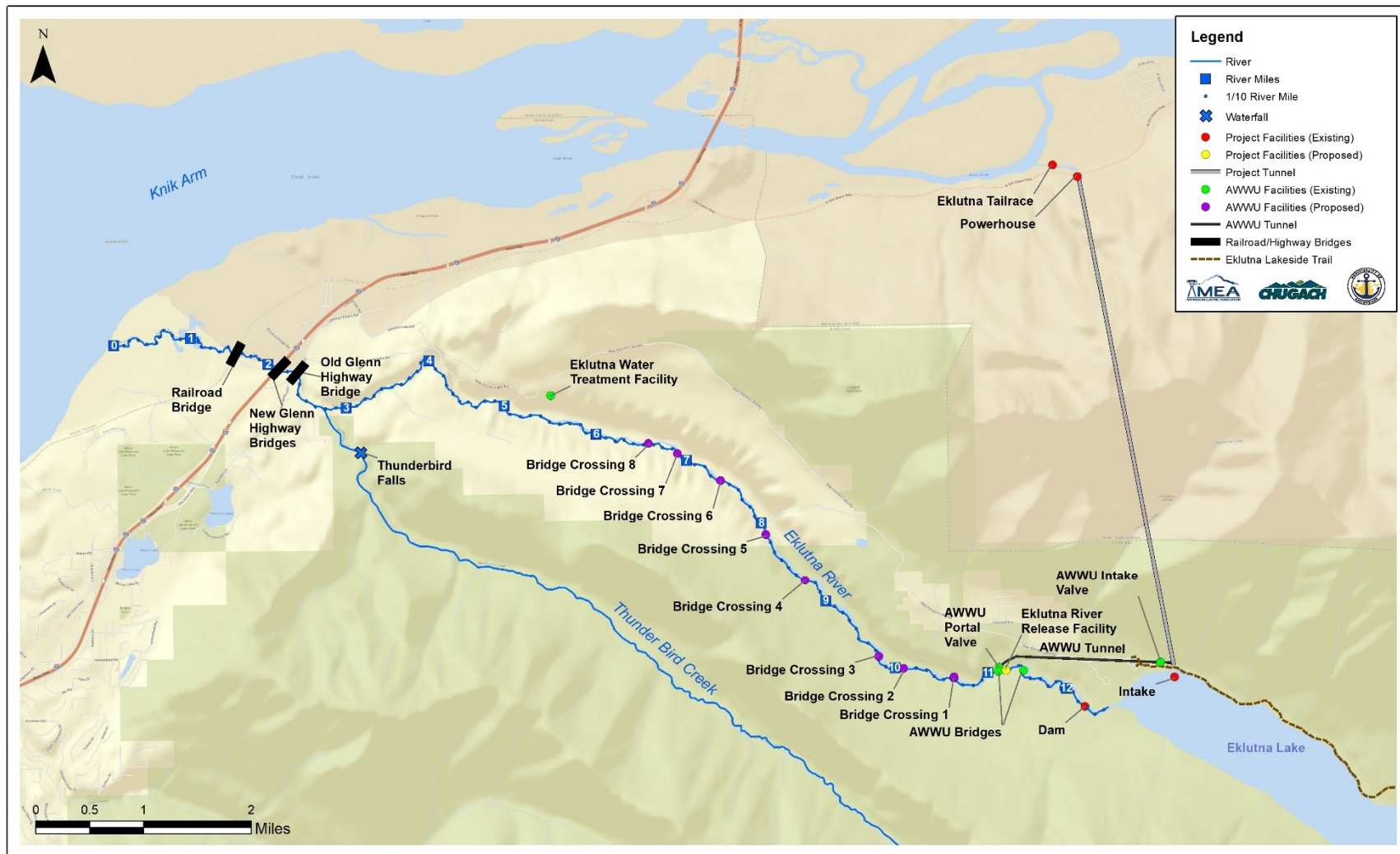


Figure 1-1. Project Overview.

## 2.0 General Description

### 2.1 General Design Criteria and Standards

General design criteria and standards for the Project include the abbreviations, survey datum, topographic mapping, references, and design standards as described below.

#### 2.1.1 Standard List of Terms and Abbreviations

AACE	Association for Advancement of Cost Engineering
AC	alternating current
ACI	American Concrete Institute
AF	acre-ft
AISC	American Institute of Steel Construction
ANSI	American National Standards Institute
ASTM	American Society of Testing and Materials
AWWA	American Water Works Association
AWWU	Anchorage Water and Wastewater Utility
BMP	Best Management Practice
CADD	computer aided design and drafting
CEA	Chugach Electric Association
cfm	cubic feet per minute
cfs	cubic feet per second
DC	direct current
DDR	Design Documentation Report
ERRV	Eklutna River Release Valve
EL.	elevation
fps	feet per second
ft	feet
HDC	Hydraulic Design Criteria
HEC	Hydrologic Engineering Center
HMI	human-machine interface
HPU	hydraulic power unit



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HVAC	heating, ventilation, and air conditioning
Hz	hertz
IESNA	Illuminating Engineering Society of North America
I/O	Input/Output
IBC	International Building Code
IEEE	Institute of Electrical and Electronics Engineers
LED	light-emitting diode
LiDAR	Light Detection and Ranging
MEA	Matanuska Electric Association
MGD	million gallons per day
MOA	Municipality of Anchorage
MSL	Mean Sea Level
NAD83	North American Datum of 1983
NAVD88	North American Vertical Datum of 1988
NEC	National Electric Code
NEMA	National Electrical Manufacturers Association
NESC	National Electrical Safety Code
NFPA	National Fire Protection Association
NGVD29	National Geodetic Vertical Datum of 1929
NVE	Native Village of Eklutna
O&M	operation and maintenance
pcf	pounds per cubic foot
PLC	programmable logic controller
psi	pounds per square-inch
RTD	Resistance Temperature Detector
SCADA	Supervisory Control and Data Acquisition
UL	Underwriters Laboratories
UPS	uninterruptible power supply
USGS	U.S. Geological Survey
V	volt

## 2.2 Survey Datum

Multiple vertical survey datums are reported in and around the main features of the Project. The following datums are identified within the Project area:

1. The MOA conducted LiDAR survey flights over the Project Area between May 12-30, 2015. The vertical datum this data is presented in is NGVD 1929 – M.O.A. 1972 Adjustment (MOA72).
2. McMillen contracted NV5 Geospatial to conduct LiDAR survey flights over the Project Area on May 28, 2020 and on May 18, 2022. The vertical datum this data is presented in is NAVD88, GEOID12B.
3. The As-Built Drawings for the dam and spillway are used for the reservoir modeling and hydraulic analysis. The drawings are titled “Eklutna Project – Alaska; Rehabilitation Pressure Tunnel” dated November 16, 1964 (USBR, Eklutna Project - Alaska. Rehabilitation Pressure Tunnel. As Built Drawings 1964). The vertical datum for these drawings is currently unknown and shall be referred to as “Local Datum” until further information is gathered.
4. The As-Built drawings of the AWWU lake diversion tunnel and pipeline are utilized as part of the design of the proposed facility. The drawings are titled “Eklutna Water Project” dated January 2, 1986 (AWWU 1986). The vertical datum used in these drawings is the GAAB Post-Quake, USC&GS, MSL, 1972. As currently understood this datum is identical to and used interchangeably with MOA72.
5. The USGS gauge station 15278000 Eklutna Lk Nr Palmer AK is reported by USGS to be based on the NGVD29. Discussions with USGS revealed that this is not necessarily the case, rather the gauge is tied to a brass cap set on the Eklutna Dam spillway wall.

The survey datum that will be utilized as the basis of this Project and the basis of all elevations within this report is NAVD88 GEOID 12B in terms of elevation with respect to Mean Sea level (MSL), US Foot. A conversion was determined from each of the unique project datums to correct to NAVD88 for the purposes of this investigation (McMillen 2022). The calculated correction factors between vertical datums used as part of the Eklutna Project is presented in Table 2-1.

Table 2-1. Eklutna Vertical Datum Conversion.

Vertical Datum	Conversion to NAVD88 (ft)	Notes
NAVD88	+0.00	
USGS Gauge	+3.263	Based on Staff Gauge Measurements at Dam
Local Datum	+3.593	Based on 2003 Survey at Dam
MOA72	+6.339	Based on 1987 Survey at Dam
	+6.520	Based on 2015 MOA LiDAR report

## 3.0 Existing Infrastructure

### 3.1 Eklutna Dam

The existing dam is located approximately 1,400 feet downstream of the natural outlet of Eklutna Lake. It is an earth and rock fill embankment dam with a crest length of 815 feet, a crest width of 30 feet, and a crest elevation (El.) of 894.6 feet (NAVD88) (USBR 1967). The crest of the dam is finished with crushed rock material to form a stable road surface, as shown in Figure 3-1. The dam allows for storage of water in Eklutna Lake for use throughout the year for power generation and water supply.



**Figure 3-1. Dam Crest and Ungated Overflow Spillway, Looking North.**

An ungated overflow spillway is incorporated within the middle of the dam. The reinforced concrete spillway consists of an inlet structure 105.54 feet long, a rectangular conduit section 83.80 feet long, a chute 77.66 feet long, and a stilling basin 89.00 feet long. The inlet structure contains a concrete overflow crest with a width of 18-feet and a crest elevation of El. 874.59 ft msl (NAVD88). The floor of the spillway is at El. 855.59 ft msl (NAVD88) for a total crest height of 19 feet. The design discharge capacity of the spillway is 3,315 cfs which corresponds to a reservoir water surface elevation of El. 888.39 ft msl (NAVD88). A cross sectional drawing of the structure is presented in Figure 3-2 with photographs of the spillway provided in Figure 3-3.

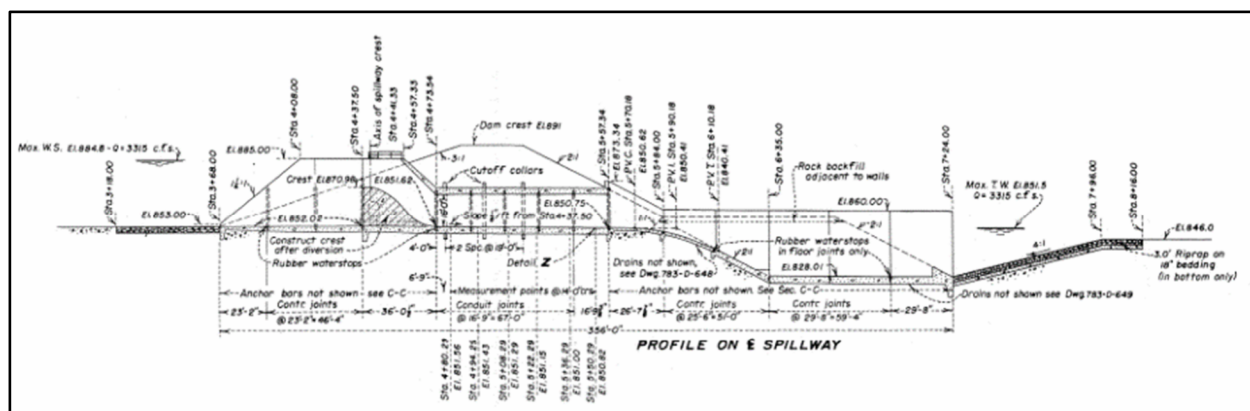


Figure 3-2. Cross Sectional View of Eklutna Dam Spillway.



Figure 3-3. Top View of Spillway (Left). Upstream Face of Spillway (Right).

## 4.0 Project Infrastructure and Operation

### 4.1 Spillway Infrastructural Modifications

The proposed modification to the Eklutna Dam Spillway involves the removal of the existing concrete overflow crest, installation of gate guides and sill plate, and installation of a fixed wheel gate, hoist, and hoist support structure. The concrete overflow crest currently provides stability to the concrete training walls; thus, removal of this element will introduce a new loading condition on the walls that must be addressed as part of the proposed scope of work. The modifications to address this new loading condition involve bracing the spillway training walls on the upper deck, which will also serve as the gate hoist platform. During the development of the design, the Alaska Department of Natural Resources Dam Safety division will be closely collaborated with to ensure the dam is protected during construction and operation of the new spillway gate. Details of the structural analysis of the existing spillway training walls under the new loading condition are provided in Appendix C. A section of the proposed infrastructural elements within the spillway is presented in Figure 4-1.

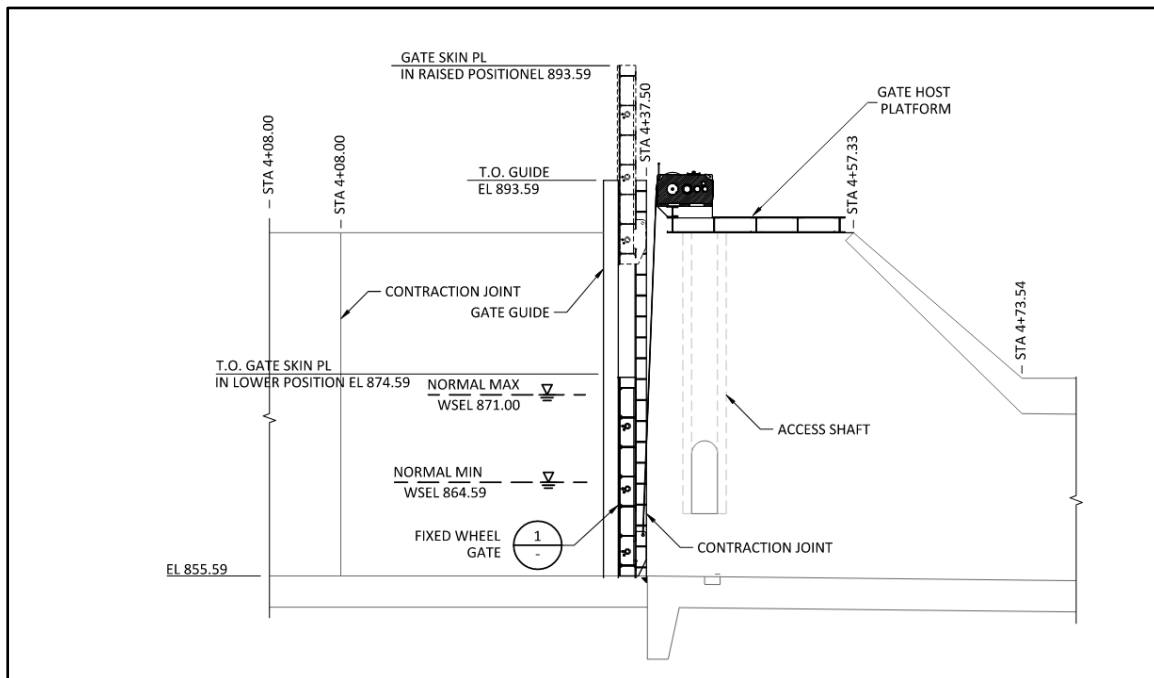


Figure 4-1. Spillway Modifications – Cross Section.

The gate hoist platform will consist of five W18x71 beams anchored across the span of the spillway. This platform will double as the support platform for the hoisting equipment for the gate. A new security fence will be located along the perimeter of the gate hoist platform and spillway upper deck to restrict public access and protect against theft or vandalism.



## 4.2 Fixed Wheel Gate

To control the flow within the spillway for regulated spill events or for future channel maintenance flow purposes, a fixed wheel gate will be implemented at Eklutna Dam. A fixed wheel gate is a type of vertical lift gate which utilizes wheels located on a fixed axle to transmit loads into the guides and supporting structure. These gates are commonly used in dams and other hydraulic structures due to their reduced friction requiring smaller hoisting mechanism and them being fairly economical to fabricate. A representative photograph of two fixed wheel gates at an existing dam intake is provided in Figure 4-2.

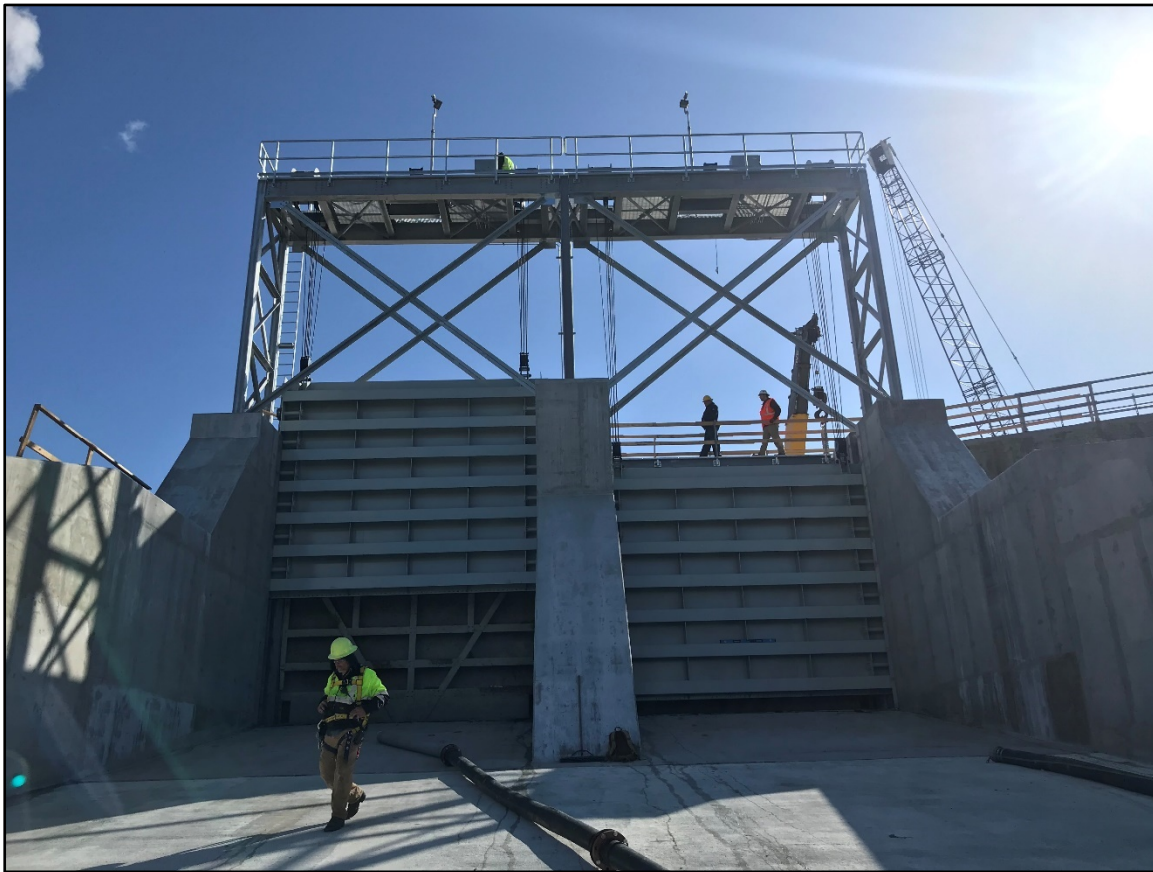


Figure 4-2. Fixed Wheel Gates at Dam Intake.

The fixed wheel gate would be lifted by a wire rope hoisting mechanism located on the proposed hoist platform on top of the spillway. The gate would be actuated locally via push button controls or remotely via the SCADA network. A stage-discharge relationship would be developed for the gate such that the flow rate released into the river may be calculated as a function of the gate opening position and the water surface elevation of the lake.

## 5.0 Structural Design Criteria

The structural design criteria apply to all design procedures to be implemented during the Project design. Structural design considerations listed in this section include material selection and design requirements that are intended to be incorporated into the Project. Structural Calculations may be found in Appendix A.

### 5.1 Applicable Codes and Standards

The structural design of the gate and hoist platform will conform to the following reports, criteria, and industry codes and standards. The applicable version of each document is the latest edition in force unless noted otherwise. References to the specific codes and standards will be included in the applicable technical specifications as the final design documents are prepared.

**Table 5-1. Structural Codes and Standards.**

Number	Description
<b>Building Code</b>	
ASCE 7	ASCE 7/SEI 7-16 Minimum Design Loads for Buildings and Other Structures, 2016
<b>Design Standards and References</b>	
ACI 318	Building Code Requirements for Structural Concrete, 2014
AISC 360	Specification for Structural Steel Buildings, 2016
ASIC SCM	Steel Construction Manual, 14th Edition
AWS D1.1	Structural Steel Welding Code – Steel
USACE EM 1110-2-2107	Design of Hydraulic Steel Structures

### 5.2 Materials

Materials and standards for concrete, concrete-related materials, structural steel, and miscellaneous metals for the Project are indicated in Table 5-2.



**Table 5-2. Structural Materials and Design Standards.**

Component	Material and Standard
<b>Concrete and Related</b>	
Existing Concrete	Compressive Strength (static), $f'_{ce} = 3000$ psi
Adhesive Anchors	Two component epoxy, ASTM C-881 (with SS threaded anchor rod or reinforcing dowels)
<b>Structural Steel</b>	
Wide Flange Shapes	ASTM A992, Grade 50ksi
M, C, L shapes	ASTM A572, Grade 50ksi
Tube (HSS)	ASTM A500, Grade C. (46ksi Round / 50ksi Rect.)
Plates and Bars	ASTM A572, Grade 50ksi
High-Strength Bolts	ASTM F3125, Grade A325
Anchor Rods	ASTM F1554, Grade 50ksi (threaded) A193, Type 316 Stainless Steel
<b>Miscellaneous Metals</b>	
Guardrail and Grating	Galvanized steel

### 5.3 Design Loads

The general load types considered in the design of the structure are summarized in Table 5-3.

**Table 5-3. Structural Design Loads.**

Load Type	Description
Dead	Self-weight of all permanent structural elements as well as super-imposed weight of permanent mechanical and electrical equipment and piping.
Live	Temporary loads produced by the use and occupancy of the building or structure (excluding environmental loads). This load type includes those produced by movable objects and loads on guardrails, fixed ladders, and handrail systems by maintenance personnel.
Wind	Wind loads will be applied to all solid surfaces of the access platform and framing.

Seismic	Inertial forces produced by ground motion during a seismic event. The effects of this load type will be approximated based on the provisions of ASCE7.
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### 5.3.1 Design Basis for Hoist Platform

Design of the access platform framing, guardrail, and connections will satisfy Load Factor and Resistance Design (LRFD) methodology by providing the required strength defined by the equation below:

$$U = \sum \gamma_i L_{ni} \leq \phi R_n$$

Where:

- $U$  = required strength
- $\gamma_i$  = load factors per ASCE 7
- $L_{ni}$  = load effects per structural calculations
- $\phi$  = resistance factor from ASCE 7
- $R_n$  = nominal resistance from ASCE 7

This methodology will be applied to the design of other material types in accordance with the relevant design standards and references. Refer to the structural calculations for additional information and derivation of these equation variables for specific Project features and components.

### 5.3.2 Design Basis for Fixed Wheel Gate

The proposed spillway fixed wheel gate will consist of a single steel skin plate welded to a series of WT shapes that are welded to a thick end plate. This design provides strength, stiffness and reliability. The roller wheel system is adjustable so that the wheels can be custom fit to the field conditions.

The gates will be fabricated from ASTM A709 Grade 50 steel and coated with a polyamine cure epoxy paint system. This type of steel is used in modern bridge fabrication because of its high toughness properties which resists sudden fracture. Seal retainer plates and bearing plates will be painted steel to reduce the amount of dissimilar metals in the vicinity which cause corrosion. The seal retainer bolts will be stainless steel. It is noted that some manufacturers choose to provide stainless steel wheels, axles, and embedded parts; however, carbon steel materials are allowed. Carbon steel components will be painted in order to provide corrosion resistance, and these coatings must be maintained by the Owners.

## 6.0 Mechanical Design Criteria

The mechanical design criteria apply to all design procedures to be implemented during the Project design. Mechanical design considerations listed in this section include detailing of mechanical components, material selection, and design requirements that are intended to be incorporated into the Project. Mechanical design calculations may be found in Appendix B.

### 6.1 Applicable Codes and Standards

The mechanical design will conform to the following reports, criteria, and industry codes and standards. The applicable version of each document is the latest edition in force unless noted otherwise. References to the specific codes and standards will be included in the applicable technical specifications as the final design documents are prepared.

**Table 6-1. Mechanical Codes and Standards.**

Codes & Standards	Description
ASTM	American Society of Testing and Material
ANSI	American National Standards Institute
ASME	American Society of Mechanical Engineers
ASME B16.5	Steel Pipe Flanges
AWS	American Welding Society
CMAA	Crane Manufacturers Association of America
FIST	Facilities Instructions, Standards and Techniques Manuals
MSS	Manufacturers Standardization Society of Valves and Fittings
USACE EM 1110-2-2610	Mechanical and Electrical Design for Lock and Dam Operating Equipment

### 6.2 Materials

Materials and standards for the mechanical components of the Project are indicated in Table 6-2.

**Table 6-2. Mechanical Materials and Design Standards.**

Component	Material and Standard
Wire Rope	6x37 Extra Improved Plow Steel – Stainless Steel
Gate Roller Bearings	Fabric Reinforced Composite Self Lubricated Bearing (CIP 151A)
Gate Roller Shafts	17-4 PH Stainless Steel
Cranes and Hoists	Steel with Vendor Coating Systems

## 6.3 Design Loads

### 6.3.1 Design Basis for Gate Hoist Components

USACE and ASME standards will be primarily used to guide spillway hoist and gate mechanical components. USACE EM 1110-2-2610 “Mechanical and Electrical Design for Lock and Dam Operating Equipment” and ASME B30.20 “Below the Hook Lifting Devices” are the most applicable. In general, they follow the Allowable Strength Design (ASD) method with the following design criteria:

- Normal operation load cases require that design stresses do not exceed 20% of material ultimate strength.
- Overload load cases require that design stresses do not exceed 75% of material yield strength.
- Overload load cases require that rope loads do not exceed 70% of minimum breaking strength.
- Self-lubricated bushing average bearing loads do not exceed 3,000 psi for normal operating conditions.

Off-the-shelf components will utilize their rated loads as they are assumed to have incorporated appropriate safety factors. In other words, the safety factors listed above will not be applied to off-the-shelf components.

### 6.3.2 Load Case A – Normal Operation

The normal operation load case (LCA) will be calculated based on the worst-case friction and efficiency values that would be obtained with moderate maintenance performed on the mechanical systems over the 50-year design life. This load case assumes that the hoist and gate are functioning as designed. Per the requirements of USACE EM 1110-2-2610, the hoist

system will be designed for a factor of safety of 5 for this load case. Load Case A is comparable to SG, U2 in Table 3-19 of USACE ETL-584. The normal operation loads on the hoist system are a function of the external loads applied to the gate (hydrostatic forces, gravitational forces, friction forces, etc.). To calculate the load required to lift the spillway gate (tension in hoist ropes), a free body diagram has been created. Operating loads were applied to the free body diagram and a summation of forces and moments was performed to solve for the tension in the wire ropes.

### 6.3.3 Load Case B – Maximum Allowable Motor Torque of Hoist

Load Case B (LCB) will be defined as the maximum torque of the motor applied to the hoist. Per the requirements of EM 1110-2-2610, the hoist system will be designed to not exceed 75% of the yield of the components under this loading except for wire rope, which will be limited to 70% of minimum breaking strength based on the guidance of USACE EM 1110-2-3200. This load case is not directly covered in USACE ETL 584 for the structural design of the gate. However, this load case is most comparable to SG.X3 in Table 3-19. This load case will be calculated based on breakdown torque of motor, which per NEMA MG-1 specifies at 200% of rated motor torque for a NEMA B motor.

## 6.4 Hoist Performance Criteria

### 6.4.1 Rated Load

The hoist load will be based on the loads listed below in Table 6-3. This rating is driven by the load required to lift the gate out of the slot under flow. It should be noted that the gate operation will require lifting from the sill at maximum normal water elevation. The contributions to hoist load total are listed in Table 6-3.

**Table 6-3. Basic Hoist Loads.**

Description	Load
Gate Weight, lbf	23,100
Roller Friction, lbf	20,368
Seal Friction, lbf	515
Ice and Mud, lbf	2,000
Inertial Loading, lbf	48

### 6.4.2 Gate Travel Speed

Gate travel speed will be one foot per minute, which is typical for dam or spillway gates of this size. The slow operating speed additionally will help regulate the down ramping within the river during spill events to prevent fish stranding.

### 6.4.3 Motor Sizing

The required power to operate the gates is 2 hp. A 5hp NEMA B motor was selected to be conservative at this time. The calculations to determine the motor capacity are provided in Appendix C.

### 6.4.4 Hoist Configuration

The hoist configuration will consist of a single 5-hp NEMA B motor with a motor brake driving a parallel shaft reducer with two output shafts, which drives two grooved wire rope drums through an 8-inch-diameter drive shaft supported by two pillow block roller bearings supporting each. The rope will be stainless steel rope and will require no maintenance or lubrication. Each side of the hoist will be two-part rope configuration (Figure 6-1).

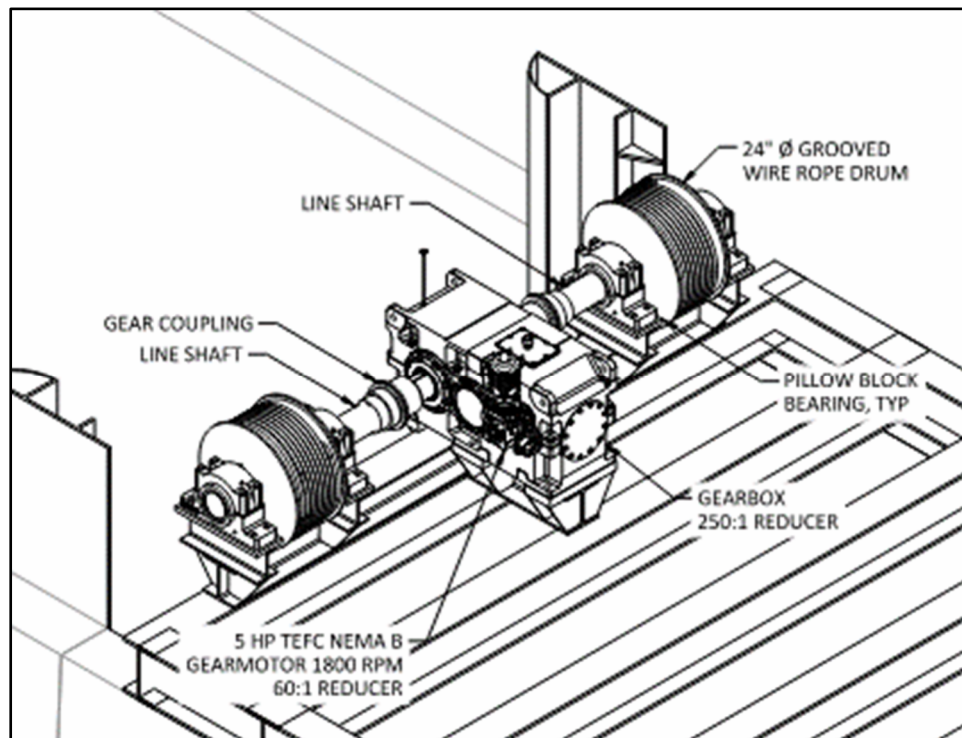


Figure 6-1. Hoist Configuration.

#### **6.4.5 Hoist Wire Rope**

The wire rope used will be 1 3/8-inch steel wire rope rated at a minimum 77,700 lbf breaking strength per ASTM A1023 – Standard Specification for Carbon Steel Wire Ropes for General Purposes.

#### **6.4.6 Overload Limit Device**

No overload limit device is required as the system will be capable of withstanding breakdown motor torque (load case B) without being damaged.

#### **6.4.7 Hoist Brake**

The motor hoist brake will be rated to be at least 250% rated motor torque. This is a higher rating than USACE EM 1110-2-2610 chapter 2-2b recommends but will ensure that motor cannot overpower the brake if erroneously engaged. A disk type brake will be used for its simplicity, robustness, effectiveness and ease of maintenance.

## 7.0 Electrical Design

The electrical design criteria apply to all design procedures implemented during the Project design. Electrical design considerations listed in this section include detailing of electrical components, material selection, and design requirements that are intended to be incorporated into the Project.

### 7.1 Applicable Codes and Standards

Applicable codes and standards are listed in Table 7-1.

**Table 7-1. Electrical Codes and Standards**

Code	Standard
NFPA 70	National Electrical Code
NFPA 70E	National Electrical Safety Code (NESC)
NFPA 110	Standard for Emergency and Standby Power Systems
IEEE	Institute of Electrical and Electronics Engineers
NEMA	National Electrical Manufacturers Association

NFPA – National Fire Protection Association

### 7.2 Materials

Materials are summarized in Table 7-2.

**Table 7-2. Electrical Materials**

Material	Standard
Circuit Breakers	NEMA AB 1, UL 489
Switches	NEMA KS 1, UL 98
Terminal Blocks	UL 1059
Power Conductors/Cable	NEMA WC 70, UL 44, UL 83, UL 854
Instrumentation Cable	NEMA WC 57, UL 13, UL 44
Boxes and Enclosures	NEMA 250



Material	Standard
Raceway	NEMA C80.1, NEMA C80.6, NEMA RN 1, UL 6, UL 360, UL 514B, UL 651, UL 1242
Standby Generators	NEMA MG 1, UL 142, UL 508, UL 1236, UL 2200
Transfer Switches	NEMA ICS 2, UL 1008
Grounding and Bonding Equipment	UL 467

NEMA – National Electrical Manufacturers Association

### 7.3 Design Loads

### 7.4 Power Distribution

The gate hoist motor will run on single phase 240 VAC and will be served from a 240V panelboard. A surge protective device will be provided at 240V and 120V to protect the system from transient surges. Power will be provided to the dam via buried 7,200V cable from the nearest transmission line located approximately 2,500 feet north of the site. A 7,200V / 240VAC single phase stepdown transformer will be located at the right abutment of the dam.

### 7.5 Process Instrumentation and Control

A SCADA system will be implemented to supervise and control the Eklutna Dam spillway gate hoist and to monitor water surface elevation within Eklutna Lake using the level transducer installed as part of the prior dam outlet modification work. The SCADA system will consist of a metal backplane in a local control panel with components mounted to it. This includes PLC and I/O modules, UPS, control power supply and distribution block, control relays, terminal blocks, overcurrent protection, digital communication hardware, and other required appurtenances. It is presumed that datalogging will be performed on the Eklutna Power Plant SCADA system. If autonomous and more continuous datalogging is determined to be required, a datalogging-capable HMI will be specified in addition to the above components.

A fiber optic communication network will be implemented between all systems of the Project via a fiber optic infrastructure. The dam outlet gate and level transducer will connect to a fiber optic communication network between Eklutna Dam, the Eklutna River Release Facility, and the Eklutna Power Plant. Digital communication protocol will be Ethernet/IP over single-mode fiber optic cable (OS2). All new segments of installation will consist of fiber optic cable that will be all-dielectric, loose-tube buffered, with water-repellant gel-fill and jacket rated for direct-bury. For adequate spares to mitigate strand failure, 24-strand cable is proposed. The

cable will be installed in HDPE SDR-11 smooth wall conduit. The fiber optic line will be brought to the LCP housed fiber connector housing for termination, then patched to the managed Ethernet switch in a loop configuration. All strands of all fiber optic cables shall be terminated and certified to the OTDR and OLTS tests. The Ethernet switch will connect the PLC to the communication system.

Field instrumentation consists of the following, as provided in Table 7-3.

**Table 7-3. Field Instrumentation**

Instrument Tag ISA)	Instrument Type	Parameters	Control Loop
LE-100	Pressure Transducer	Water Level Monitoring, 0-30 ft H <sub>2</sub> O range	Measure Water Level in Stilling Well within Eklutna Dam Spillway
HOI-100	Electric Motor Hoist	Hoist Position	Measure Status of Encoder and Position of Gate

## 8.0 References

AWWU. 1986. *Eklutna Water Project. As Built Drawings.*

McMillen. 2022. *Eklutna Vertical Datum Memorandum .*

USBR. 1964. *Eklutna Project - Alaska. Rehabilitation Pressure Tunnel. As Built Drawings .*

—. 1967. *Rehabilitation of Eklutna Project Features following Earthquake of March 1964. A Supplement to Eklutna Dam, Tunnel and Powerplant Technical Record of Design and Construction.*

## **Appendix A. 15% Design Drawings**

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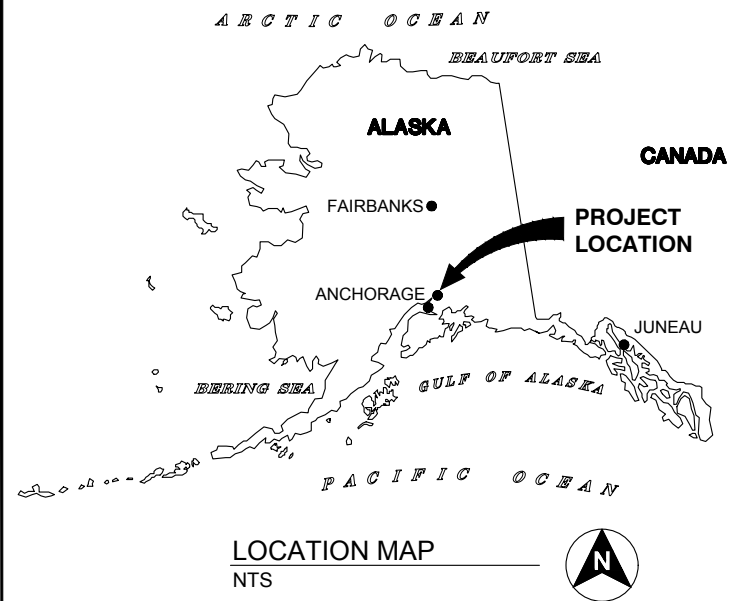
EKLUTNA FISH & WILDLIFE PROJECT  
EKLUTNA DAM SPILLWAY MODIFICATIONS  
ANCHORAGE, ALASKA

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15% DESIGN  
APRIL 2024

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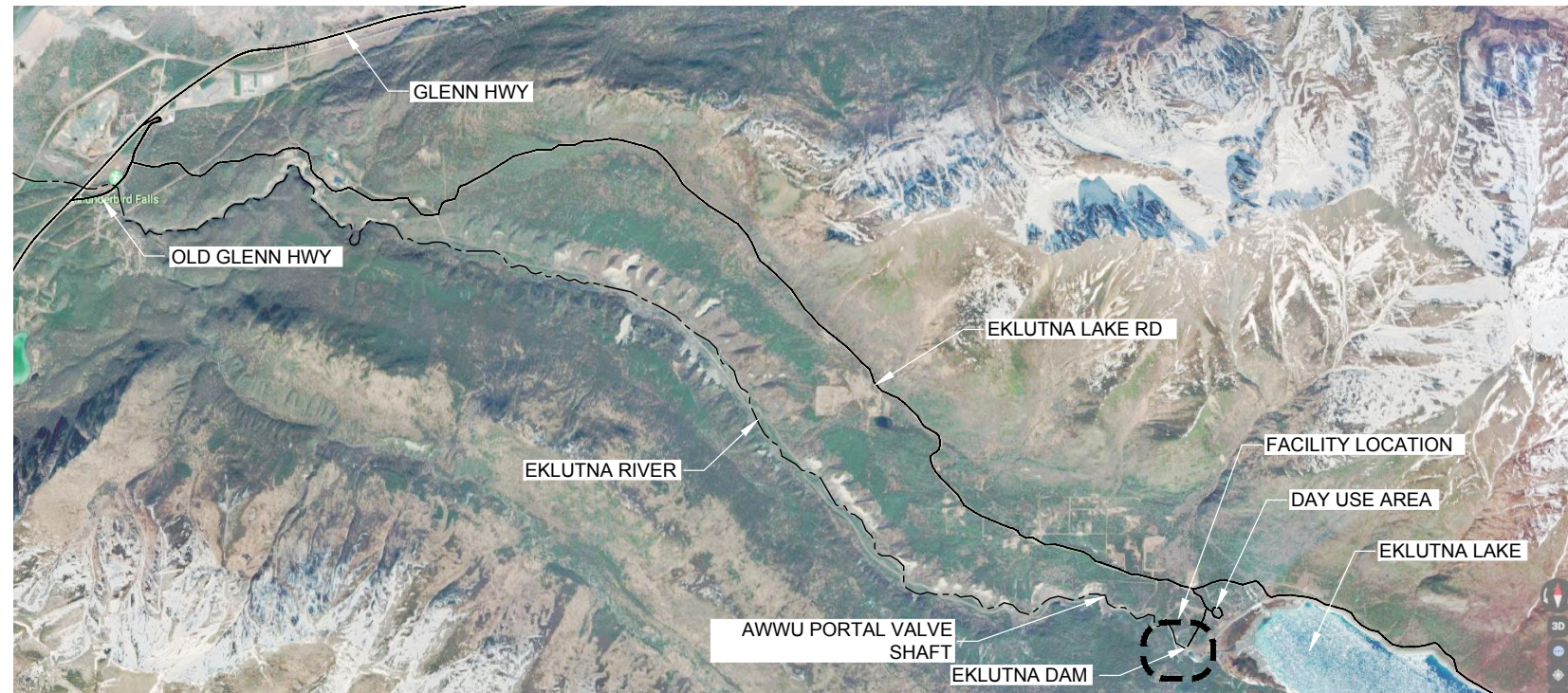




# EKLUTNA FISH & WILDLIFE PROJECT

## EKLUTNA DAM SPILLWAY MODIFICATIONS

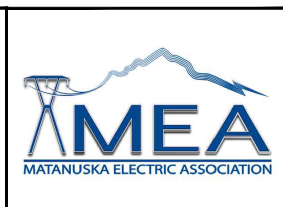
### 15% DESIGN



PRELIMINARY  
NOT FOR CONSTRUCTION

A	04/26/24	SPE	15% DESIGN	
REV	DATE	BY		DESCRIPTION

WARNING  
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IF THIS BAR DOES NOT  
MEASURE 1" THEN  
DRAWING IS NOT TO SCALE



EKLUTNA FISH & WILDLIFE PROJECT
EKLUTNA DAM SPILLWAY MODIFICATIONS
LOCATION MAP, VICINITY MAP, AND FACILITY MAP

DESIGNED	S. ELLENSON
DRAWN	F. HABER
CHECKED	J. BOAG
PROJECT DATE	04/26/24

DRAWING  
**G001**



DRAWING INDEX	
DRAWING NO.	SHEET TITLE
GENERAL	
--	COVER
G001	LOCATION MAP, VICINITY MAP, AND FACILITY MAP
G002	DRAWING INDEX
G003	STANDARD ABBREVIATIONS
G004	STANDARD SYMBOLS
G005	INSTRUMENTATION AND EQUIPMENT LEGEND
DEMOLITION	
D001	DEMOLITION KEY PLAN
D100	EKLUTNA DAM SPILLWAY DEMOLITION PLAN, SECTIONS, AND DETAILS
D101	EKLUTNA DAM SPILLWAY DEMOLITION PHOTOS
STRUCTURAL	
GS001	STRUCTURAL GENERAL NOTES
GS002	STRUCTURAL STANDARD DETAILS
S100	EKLUTNA DAM SPILLWAY STRUCTURAL PLANS
S101	EKLUTNA DAM SPILLWAY STRUCTURAL SECTIONS
MECHANICAL	
GM001	MECHANICAL GENERAL NOTES
GM002	MECHANICAL STANDARD DETAILS
M100	EKLUTNA DAM SPILWAY MECHANICAL PLAN AND SECTIONS
M101	EKLUTNA DAM SPILLWAY MECHANICAL DETAILS
ELECTRICAL	
GE001	ELECTRICAL ABBREVIATIONS AND DEVICE INDEXES
GE002	ELECTRICAL STANDARD SYMBOLS 1
GE003	ELECTRICAL STANDARD SYMBOLS 2
E001	ELECTRICAL TRANSMISSION & COMMUNICATIONS UPGRADES PLAN
E100	EKLUTNA DAM SPILLWAY ELECTRICAL PLAN, SECTION, AND DETAILS

PRELIMINARY  
NOT FOR CONSTRUCTION

A	04/26/24	SPE	15% DESIGN	
REV	DATE	BY		DESCRIPTION

WARNING

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1475 Shoreline Dr. Ste 100, Boise, ID 83702 | (208) 342-4214 | [mcmillen.com](http://mcmillen.com)

POWERING ALASKA'S FUTURE

MATANUSKA ELECTRIC ASSOCIATION

EKLUTNA FISH & WILDLIFE PROJECT	
EKLUTNA DAM SPILLWAY MODIFICATIONS	
DRAWING INDEX	

DESIGNED	S. ELLENSON
DRAWN	F. HABER
CHECKED	J. BOAG
PROJECT DATE	04/26/24

DRAWING

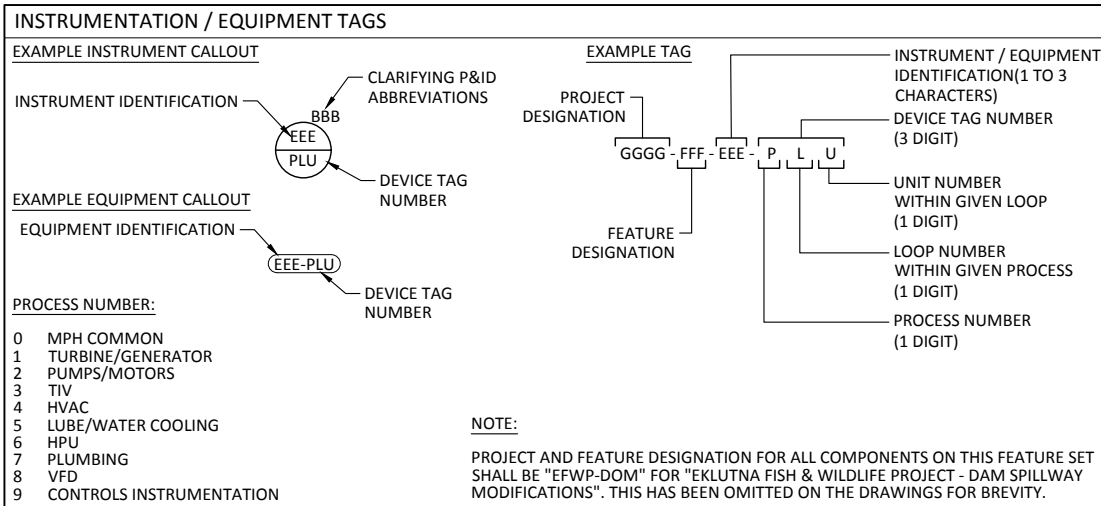
G002

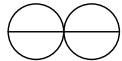
























A/C	AIR CONDITIONING	CKT	CIRCUIT	EXT	EXTERIOR, EXTERNAL, EXTENSION	I	INSTRUMENTATION (DWG DISCIPLINE)	N	NORTH, NEUTRAL	RESIL	RESILIENT	U	URINAL
A/E	ARCHITECT/ENGINEER	CL	CENTERLINE, CLASS, CLOSE			ID	INSIDE DIAMETER, INTERIOR DIMENSION	NA	NOT APPLICABLE	RET	RETAINING, RETURN	UG	UNDERGROUND
A	ARCHITECTURAL (DWG DISCIPLINE), AMP	CLR	CLEAR	F TO F	FACE TO FACE	IE	INVERT ELEVATION	NAT	NATURAL	REV	REVISION, REVERSE	ULT	ULTIMATE
AB	ANCHOR BOLT	CMH	COMMUNICATION MANHOLE	FAB	FABRICATE	IF	INSIDE FACE	NC	NORMALLY CLOSED	RFL	REFLECTED, REFLECTOR	UNFN	UNFINISHED
ABC	AGGREGATE BASE COURSE	CMU	CONCRETE MASONRY UNIT	FBO	FURNISHED BY OWNER	IH	INTAKE HOOD	NEG	NEGATIVE	RGS	RIGID GALVANIZED STEEL	UNO	UNLESS NOTED OTHERWISE
ABAN	ABANDON	CO	CLEAN OUT, CONCRETE OPENING	FC	FLUSHING CONNECTION	IMP	IMPACT	NF	NEAR FACE, NON-FUSED	RH	RELIEF HOOD, RIGHT HAND, RELATIVE	UTIL	UTILITY
AC	ALTERNATING CURRENT	COL	COLUMN	FCA	FLANGED COUPLING ADAPTER	IN	INCH	NG	NATURAL GAS		HUMIDITY		
ACST	ACOUSTIC	COM	COMMON	FCV	FIXED CONE VALVE	INC	INCLUDE, INCANDESCENT	NIC	NOT IN CONTRACT	RL	REQUIRED LAP	V	VENT, VELOCITY, VOLT
AD	ADDENDUM, AREA DRAIN	COMB	COMBINATION	FD	FLOOR DRAIN	INF	INFLUENT	NO	NORMALLY OPEN, NUMBER	RND	ROUND	VA	VOLT AMPERE
ADDL	ADDITIONAL	COMM	COMMUNICATION	FDC	FLEXIBLE DUCT CONNECTION	INSTR	INSTRUMENTATION	NOM	NOMINAL	RNG	RENEWABLE NATURAL GAS	VAC	VACUUM
ADH	ADHESIVE	COMP	COMPOSITION, COMPRESSIBLE, COMPOSITE	FDR	FEEDER	INSUL	INSULATION	NPS	NOMINAL PIPE SIZE	RO	ROUGH OPENING	VAR	VARNISH, VARIABLE, VOLT AMPERES REACTIVE
ADJ	ADJUSTABLE, ADJACENT	CONC	CONCENTRIC, CONCRETE	FE	FLANGED END	INT	INTERIOR, INTERSECTION	NPT	NATIONAL PIPE THREAD	ROW	RIGHT-OF-WAY	VB	VAPOR BARRIER, VINYL BASE, VALVE BOX
AF	AMP FRAME, AMP FUSE	CONN	CONNECTION	FEC	FIRE EXTINGUISHER CABINET	INTR	INTERMEDIATE, INTERIOR	NS	NEAR SIDE	RPM	REVOLUTIONS PER MINUTE	VC	VERTICAL CURVE
AFF	ABOVE FINISH FLOOR	CONST	CONSTRUCTION	FEXT	FIRE EXTINGUISHER	INV	INVERT	NTS	NOT TO SCALE	RR	RAILROAD	VCT	VINYL COMPOSITION TILE, VERTICAL CENTERLINE
AFG	ABOVE FINISH GRADE	CONT	CONTINUOUS, CONTINUED	FF	FAR FACE, FACTORY FINISH, FLAT FACE	IPS	IRON PIPE SIZE	NWL	NORMAL WATER LEVEL	RT	RIGHT		
AGGR	AGGREGATE	COORD	COORDINATE	FG	FINISHED GRADE	IPT	INTERNAL PIPE THREAD					VEL	VELOCITY
AIC	AMPS INTERRUPTING CAPACITY	CORR	CORROSIVE, CORRUGATED	FIG	FIGURE	IRR	IRRIGATION			S	SOUTH, SINK, STRUCTURAL (DWG DISCIPLINE)	VENT	VENTILATION
ALIG	ALIGNMENT	CP	CHECKER PLATE, CONTROL POINT	FH	FIRE HYDRANT	ISO	ISOMETRIC			SA	SUPPLY AIR	VERT	VERTICAL
ALUM	ALUMINUM	CPLG	COUPLING	FIN	FINISH					SAN	SANITARY	VS	VERSES, VAPOR SEAL
ALT	ALTERNATE, ALTITUDE	CSK	COUNTERSINK	FL	FLOW, FLOW LINE					SCPD	OVER CURRENT PROTECTION DEVICE	VOL	VOLUME
AMB	AMBIENT	CTR	CENTER	FLEX	FLEXIBLE					OD	OUTSIDE DIAMETER	VPC	VERTICAL POINT OF CURVATURE
ANC	ANCHOR	CTRL	CONTROL	FLG	FLANGE					OH	OVERHEAD	VPI	VERTICAL POINT OF INTERSECTION
AP	ACCESS PANEL	CU	COPPER, CUBIC	FLOR	FLUORESCENT					OPNG	OPENING	VPT	VERTICAL POINT OF TANGENCY
APRX	APPROXIMATE	CW	CLOCKWISE	FLR	FLOOR					OPP	OPPOSITE	VTR	VENT THROUGH ROOF
APVD	APPROVED	CY	CUBIC YARD	FLS	FLASHING, FLUSH					OPT	OPTIONAL	VWC	VINYL WALL COVERING
ARCH	ARCHITECTURAL			FND	FOUNDATION					ORD	OVERFLOW ROOF DRAIN		
ASSY	ASSEMBLY	d	PENNY (NAIL MEASURE)	FNC	FENCE					ORIG	ORIGINAL	W/	WITH
AT	AMP TRIP	D	DEEP, DIFFUSER	FO	FINISHED OPENING					OVFL	OVERFLOW	W/O	WITHOUT
ATM	ATMOSPHERE	DB	DUCT BANK, DECIBEL, DRY BULB	FOB	FLAT ON BOTTOM					OVHG	OVERHANG	W	WATT, WEST, WIDE, WINDOW, WIRE, WIDE
AUTO	AUTOMATIC	DBA	DEFORMED BAR ANCHOR	FOC	FACE OF CONCRETE, FACE OF CURB, FIBER					OZ	OUNCE		FLANGE BEAM
AUX	AUXILIARY	DBL	DOUBLE		OPTIC CABLE							WC	WATER CLOSET, WATER COLUMN
AVE	AVENUE	DC	DIRECT CURRENT	FOF	FACE OF FINISH					P	PAINT, PROCESS (DWG DISCIPLINE)	WD	WIDTH
AVG	AVERAGE	DEG	DEGREE	FOM	FACE OF MASONRY					PAR	PARALLEL, PARAPET	WF	WIDE FLANGE, WASH FOUNTAIN
AWG	AMERICAN WIRE GAGE	DEG C	DEGREE CENTIGRADE	FOS	FACE OF STUDS					PB	PANIC BAR, PULL BOX	WG	WIRE GLASS, WATER GAGE
AWWU	ANCHORE WATER AND WASTEWATER UTILITY	DEG F	DEGREE FAHRENHEIT	FOT	FLAT ON TOP					PBD	PARTICLE BOARD	WH	WALL HYDRANT, WEEP HOLE
		DEMO	DEMOLITION	FPT	FEMALE PIPE THREAD					PC	POINT OF CURVE, PIECE, PRECAST	WL	WATER LEVEL
		DEP	DEPRESSED	FR	FRAME					PCC	POINT OF COMPOUND CURVATURE	WLD	WELDED
		DEPT	DEPARTMENT	FRP	FIBERGLASS REINFORCED PLASTIC					PCF	POUNDS PER CUBIC FOOT	WM	WIRE MESH
		DET	DETAIL	FS	FLOOR SINK, FAR SIDE					PCT	PERCENT	WP	WEATHERPROOF, WORKING POINT
		DI	DROP INLET, DUCTILE IRON	FT	FEET, FOOT					PE	PLAIN END	WTHP	WEATHERPROOF
		DIA	DIAMETER	FTG	FOOTING, FITTING FUR FURRED, FURRING					PED	PEDESTAL	WS	WATERSTOP, WATER SURFACE
		DIAG	DIAGONAL, DIAGRAM	FURN	FURNITURE, FURNISH					PEN	PENETRATION	WSEL	WATER SURFACE ELEVATION
		DIFF	DIFFERENTIAL, DIFFERENCE	FUT	FUTURE					PERF	PERFORATED	WT	WEIGHT, WATER TIGHT
		DIM	DIMENSION	FV	FACE VELOCITY					PERM	PERMANENT	WWF	WELDED WIRE FABRIC
		DISCH	DISCHARGE	FW	FIELD WELD, FIRE WALL					PERP	PERPENDICULAR		
		DIST	DISTANCE, DISTRIBUTION	FWD	FORWARD					PF	POWER FACTOR		
		DIV	DIVISION	FWE	FURNISHED WITH EQUIPMENT					PH	PHASE	XS	EXTRA STRONG
		DL	DEAD LOAD	FXTR	FIXTURE					PI	POINT OF INTERSECTION	XXS	DOUBLE EXTRA STRONG
		DN	DOWN							PKG	PACKAGE	XSECT	CROSS SECTION
		DP	DEPTH	G	GRILLE, GROUND, GENERAL (DWG DISCIPLINE)					PL	PLATE, PROPERTY LINE		
		DS	DOWN SPOUT	GA	GAGE (METAL THICKNESS)					PLB	PLUMBING	YH	YARD HYDRANT
		DT	DOUBLE TEE, DRIP TRAP ASSEMBLY	GAL	GALLON					PLF	POUNDS PER LINEAR FOOT	YS	YIELD STRENGTH
		DUP	DUPLICATE	GALV	GALVANIZED					PNEU	PNEUMATIC		
		DWG	DRAWING	GB	GRADE BREAK					POS	POSITIVE, POSITION		
		DWL	DOWEL	GD	GUARD					PP	POLYPROPYLENE, POWER POLE		
				GEN	GENERAL					PRC	POINT OF REVERSE CURVATURE		
		E	EAST, ELECTRICAL (DWG DISCIPLINE)	GFCI	GROUND FAULT CIRCUIT INTERRUPTER					PREF	PREFINISHED		
		EA	EACH, EXHAUST AIR	GL	GLASS					PREFAB	PREFABRICATED		
		EC	ELECTRICAL CONTRACTOR	GP	GUY POLE					PRELIM	PRELIMINARY		
		ECC	ECCENTRIC	GR	GRADE					PREP	PREPARE		
		EDB	ELECTRICAL DUCT BANK	GRND	GROUND					PRES	PRESSURE		
		EE	EACH END	GRTG	GRATING					PROP	PROPERTY		
		EF	EACH FACE	GT	GREASE TRAP					PROT	PROTECTION		
		EG	EXISTING GRADE	GWB	GYPSPUM WALLBOARD					PSF	POUNDS PER SQUARE FOOT		
		EGL	ENERGY GRADE LINE	GYP	GYPSPUM HARDBOARD					PSI	POUNDS PER SQUARE INCH		
		EFF	EFFLUENT, EFFICIENCY							PSIA	POUNDS PER SQUARE INCH ABSOLUTE		
		EHH	ELECTRICAL HANDHOLE	H	HIGH					PSIG	POUNDS PER SQUARE INCH GAGE		
		EIFS	EXTERIOR INSULATION & FINISH SYSTEM	HB	HOSE BIB					PT	POINT, POINT OF TANGENCY		
		EJ	EXPANSION JOINT	HBD	HARDBOARD					PTN	PARTITION		
		EL	ELBOW, ELEVATION	HC	HANDICAPPED, HOLLOW CORE, HORIZONTAL CURVE					PVC	POLYVINYL CHLORIDE		
		ELEC	ELECTRICAL							PVMT	PAVEMENT		
		EMBD	EMBEDDED	HC	HORIZONTAL CENTERLINE					PWD	PLYWOOD		
		EMER	EMERGENCY	HDR	HEADER					PZ	PIEZOMETER		
		EMH	ELECTRICAL MANHOLE	HDW	HARDWARE								
		ENCL	ENCLOSURE	MED	MEDIUM					Q	RATE OF FLOW		
		ENGR	ENGINEER	MFR	MANUFACTURER					QTR	QUARTER		
		ENTR	ENTRANCE	MH	MANHOLE, METAL HALIDE					QTY	QUANTITY		
		EOP	EDGE OF PAVEMENT	HORIZ	HORIZONTAL					QUAL	QUALITY		
		EOW	EDGE OF WATER	HP	HIGH POINT, HORSEPOWER								
		EQ	EQUAL	HPC	HORIZONTAL POINT OF CURVATURE					R&R	REMOVE AND REPLACE		
		EQUIP	EQUIPMENT	HPS	HIGH PRESSURE SODIUM					R&S	REMOVE AND SALVAGE		
		EQUIV	EQUIVALENT	HPT	HORIZONTAL POINT OF TANGENCY					R	RADIUS, REGISTER, RISER		
		ES	EACH SIDE, EQUAL SPACE, EMERGENCY SHOWER	HR	HOOR					RA	RETURN AIR		
				HS	HEADED STUD, HIGH STRENGTH					RB	RESILIENT BASE, ROCK BERM		
		ESEW	EMERGENCY SHOWER AND EYE WASH	HSS	HOLLOW STRUCTURAL SHAPE					RCPT	RECEPTACLE		
		EST	ESTIMATE	HT	HEIGHT					RD	ROOF DRAIN		
		EW	EACH WAY, EMERGENCY EYE/FACE WASH	HV	HIGH VOLTAGE					REC	RECESS		
		EWEC	ELECTRIC WATER COOLER	HVAC	HEATING, VENTILATION & AIR CONDITIONING					RECD	RECEIVED		
		EWEF	EACH WAY, EACH FACE							RECT	RECTANGULAR		
		EWTB	EACH WAY, TOP AND BOTTOM							RED	REDUCER		
		EXC	EXCAVATION							REF	REFERENCE		
		EXH	EXHAUST							REINF	REINFORCING		
		EXIST	EXISTING							REQD	REQUIRED		
		EXP	EXPANSION, EXPOSED										
											</		

[illegible]





GENERAL INSTRUMENT OR FUNCTIONAL SYMBOLS						SPECIAL CASE INSTRUMENT OR FUNCTIONAL SYMBOLS	
	FIELD MOUNTED	PRIMARY OR PANEL MOUNTED - ACCESSIBLE (1)	PRIMARY OR PANEL MOUNTED - INACCESSIBLE (2)	SECONDARY OR MCC MOUNTED - ACCESSIBLE (1)	SECONDARY OR MCC MOUNTED - INACCESSIBLE (2)		SINGLE INSTRUMENT OR OTHER COMPONENT HAVING MULTIPLE FUNCTIONS
							RELAY INTERLOCK LOGIC - SEE SCHEMATICS OR SPECIFICATIONS FOR MORE INFORMATION
							LEVEL (FLOAT)
							LEVEL (ULTRASONIC)
INSTRUMENT							
SHARED DISPLAY SHARED CONTROL OR HMI							POWER SUPPLY (SIZE AS NOTED)
							AIR SUPPLY
INDICATING LIGHTS			N/A		N/A		PRIMARY ELECTRICAL POWER (120V / 60 HZ UNLESS INDICATED OTHERWISE)
						*	INDICATES VENDOR PACKAGE
(1) NORMALLY ACCESSIBLE TO OPERATOR (2) NORMALLY INACCESSIBLE TO OPERATOR (BEHIND-THE-PANEL)							CONTROL RELAY
							LIGHTNING SURGE ARRESTOR
							MOTOR

## SIGNAL SYSTEM INTERFACES

ANALOG  
INPUT

ANALOG  
OUTPUT

DISCRETE  
INPUT

DISCRETE  
OUTPUT

DIGITAL DATA  
SIGNAL

AA = I/O DESIGNATION (MV = MULTIVARIABLE)

YY = DIGITAL PROTOCOL

X: H-MAINTAINED/LATCHING  
M = MOMENTARY/FOLLOWER

### ANALOG I/O DESIGNATORS

CR	CHLORINE RESIDUAL
DP	DIFFERENTIAL PRESSURE
FL	FLOW
LE	LOWER EXPLOSIVE LIMIT
LV	LEVEL
MO	MANIPULATED OUTPUT
PH	ACIDITY
PO	POSITION
PR	PRESSURE
PV	PROCESS VARIABLE
SP	SPEED
TE	TEMPERATURE
TU	TURBIDITY

### DISCRETE I/O DESIGNATORS

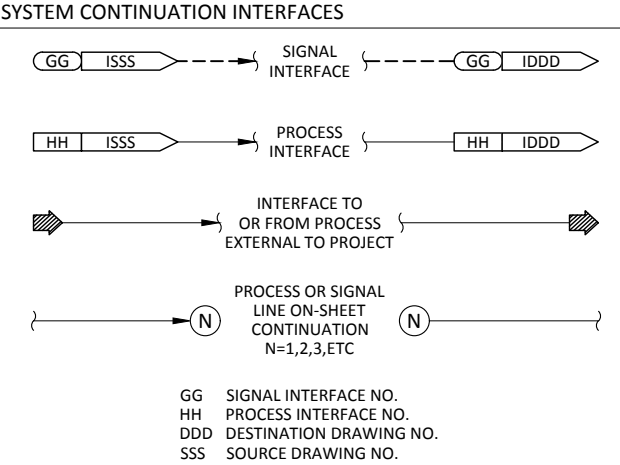
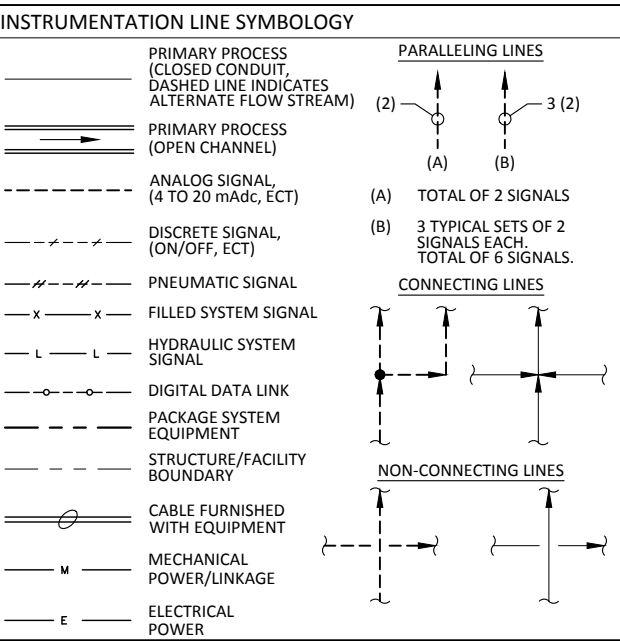
AM	AUTO-MANUAL
AU	AUTO
CL	CLOSED
EN	ENABLE
EL	POWER AVAILABLE
FA	FIRE ALARM
FW	FORWARD / REVERSE
HH	HI-HI LEVEL
HI	HI LEVEL
LL	LOW-LOW LEVEL
LO	LOW LEVEL
MN	MANUAL
OO	ON-OFF
OP	OPEN
RB	RUN BOOSTER
RC	RUN CLOSED
RE	REMOTE
RF	RUN FORWARD
RG	RUNNING
RN	RUN-STOP
RO	RUN-OPEN
RR	RUN-REVERSE
RV	REVERSE
YA	FAULT
SU	SUPERVISORY
SW	SELECTION
TR	TROUBLE

### DIGITAL PROTOCOL DESIGNATORS

DN	DEVICENET
IP	ETHERNET /IP
MB	MODBUS RTU
PB	PROFIBUS
PL	PARALLEL
SL	SERIAL
TC	MODBUS TCP

FIRST LETTER			SUCCEEDING LETTER(S)		
LETTER	MEASURED INITIATING VARIABLE	VARIABLE MODIFIER	READOUT OR PASSIVE FUNCTION	OUTPUT OR ACTIVE FUNCTION	FUNCTION MODIFIER
A	ANALYSIS (+)		ALARM		
B	BURNER, COMBUSTION				
C	CONDUCTIVITY			CONTROL	CLOSED
D	DENSITY (S.G.)	DIFFERENTIAL			
E	VOLTAGE		SENSOR (PRIMARY ELEMENT)		
F	FLOW RATE	RATIO (FRACTION)			
G	GAUGE		GLASS, GAUGE, VIEWING DEVICE	GATE	
H	HAND (MANUAL)				HIGH
I	CURRENT (ELECTRICAL)		INDICATE		
J	POWER	SCAN			
K	TIME, TIME SCHEDULE	TIME RATE OF CHANGE		CONTROL STATION	
L	LEVEL		LIGHT (PILOT)		LOW
M	MOTION	MOMENTARY			MIDDLE, INTERMEDIATE
N	TORQUE		ISOLATE	ISOLATOR	
O	USER CHOICE		ORIFICE, RESTRICTION		OPEN
P	PRESSURE (VACUUM), PNEUMATIC		POINT (TEST) CONNECTION		
Q	QUANTITY	INTEGRATE, TOTALIZE			
R	RADIATION/ RESISTANCE (ELECTRICAL)		RECORD OR PRINT		
S	SPEED, FREQUENCY	SAFETY		SWITCH	
T	TEMPERATURE			TRANSMIT	
U	MULTI VARIABLE		MULTIFUNCTION	MULTIFUNCTION	MULTIFUNCTION
V	VIBRATION, MECHANICAL ANALYSIS			VALVE, DAMPER, LOUVER	
W	WEIGHT, FORCE		WELL		
X	INTRUSION	X-AXIS			
Y	EVENT, STATE OR PRESENCE	Y-AXIS		RELAY, COMPUTE, CONVERT	
Z	POSITION, DIMENSION	Z-AXIS		DRIVER, ACTUATOR, FINAL CONTROL ELEMENT	

AC	AIR COMPRESSOR	GEN	GENERATOR
ACC	ACCUMULATOR	GSU	GENERATOR STEP-UP TRANSFORMER
ACT	ACTUATOR	GTC	GENERATOR POWER TERMINAL CABINET
AF	AIR FILTER	HB	HOSE BIB
AFD	ADJUSTABLE FREQUENCY DRIVE	HMI	HUMAN-MACHINE INTERFACE
AH	AIR HANDLING UNIT	HOI	HOIST/CRANE
ARC	ARC PLENUM AND EXHAUST DUCT	HPU	HYDRAULIC POWER UNIT
ATS	AUTOMATIC TRANSFER SWITCH	HTR	HEATER
BAT	BATTERY	INV	INVERTER
BC	BATTERY CHARGER	LCP	LOCAL CONTROL PANEL
BRG	BEARING	LCS	LOCAL CONTROL STATION
BRK	BREAKER	LPU	LUBRICATING OIL PUMP CONTROL UNIT
CAM	CAMERA	MB	METER BASE
CSE	COMBINATION SERVICE ENCLOSURE	MC	MECHANICAL COUPLING
CV	CHECK VALVE	MCC	MOTOR CONTROL CENTER
D	DAMPER	MCP	MAIN CONTROL PANEL
DCU	DISTRIBUTED CONTROL UNIT	MES	MANAGED ETHERNET SWITCH
DS	DISCONNECT	MOV	MOTOR OPERATED VALVE
EAP	ENGINEERING ACCESS POINT	MS	MOTOR STARTER
ECP	ENVIRONMENTAL CONTROL PANEL (HVAC)	MTR	MOTOR
EEW	EMERGENCY EYEWASH STATION	MTS	MANUAL TRANSFER SWITCH
EF	EXHAUST FAN	NET	NETWORK / COMMUNICATIONS RACK
EXC	EXCITER	OWS	OIL WATER SEPARATOR
FAS	FIRE ALARM SYSTEM	P	PUMP
FD	FLOOR DRAIN	PB	PANELBOARD / LOAD CENTER
FIL	FILTER	PCP	PLANT CONTROL PANEL
FOR	FIBER OPTIC REPEATER	PCU	POWER CONTROL UNIT
FOT	FIBER OPTIC TRANSCEIVER	PFL	PRE-FILTER
FPP	FIBER PATCH PANEL / CONNECTOR HOUSING	PFL	PROGRAMMABLE LOGIC CONTROLLER
G	GATE	PRV	PRESSURE REDUCING VALVE
GBK	GENERATOR BRAKE	PS	POWER SUPPLY / ISOLATOR / CONVERTER



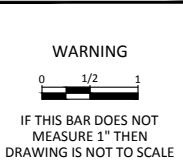
P&ID ABBREVIATIONS	
AC	ALTERNATING CURRENT
AM	AUTO-MANUAL
COD	CHEMICAL OXYGEN DEMAND
DEV	DEVIATION
DC	DIRECT CURRENT
DCS	DISTRIBUTED CONTROL SYSTEM
ECS	ENVIRONMENTAL CONTROL SYSTEM (HVAC)
EPO	EMERGENCY POWER OFF
FOC	FIBER OPTIC CABLE
FOS	FAST-OFF-SLOW
FOSA	FAST-OFF-SLOW-AUTO
FOSR	FAST-OFF-SLOW-REMOTE
HI	HIGH
HML	HIGH-MID-LOW
HOA	HAND-OFF-AUTO
HOR	HAND-OFF-REMOTE
ISR	INTRINSICALLY SAFE RELAY
LEL	LOWER EXPLOSIVE LIMIT
LO	LOW
LOR	LOCAL-OFF-REMOTE
LOS	LOCKOUT STOP
LR	LOCAL-REMOTE
MC	MODULATE-CLOSE
MOA	MANUAL-OFF-AUTO
MSC	MANUFACTURER SUPPLIED CABLE
NC	NORMALLY CLOSED
NO	NORMALLY OPEN
OC	OPEN-CLOSE(D)
OCA	OPEN-CLOSE-AUTO
OCR	OPEN-CLOSE-REMOTE
OI	OPERATOR INTERFACE
OO	ON-OFF
OOA	ON-OFF-AUTO
OOR	ON-OFF-REMOTE
ORP	OXIDATION REDUCTION POTENTIAL
OSC	OPEN-STOP-CLOSE
PC	PERSONAL COMPUTER
PCS	PLANT CONTROL SYSTEM
pH	HYDROGEN ION CONCENTRATION
PID	PROPORTIONAL INTEGRAL DERIVATIVE CONTROL
POT	POTENTIOMETER
RC	RUN CLOSE
RO	RUN OPEN
RL	RAISE-LOWER
RM	REMOTE MULTIPLEXING MODULE
RSL	RAISE-STOP-LOWER
RVSS	REDUCED VOLTAGE SOLID-STATE STARTER
SCADA	SUPERVISORY CONTROL AND DATA ACQUISITION
SEL	SELECT
SET	SET POINT
SF	SLOWER-FASTER
SHC	SODIUM HYPOCHLORITE
SR	START-RESET
SS	START-STOP
SSC	SUPERVISORY SET POINT CONTROL
ST	START
SW	SEAL WATER
TC	THERMOCOUPLE
TCR	TOTAL CHLORINE RESIDUAL
TEMP	TEMPERATURE
TSP	TWISTED SHIELD PAIR
TURB	TURBIDITY
VHC	VOLATILE HYDROCARBONS
VIB	VIBRATION
VSP	VENDOR SUPPLIED PANEL
VTO	VENT TO OUTSIDE
WSEL	WATER SURFACE ELEVATION

NOTES:

1. FOR MECHANICAL ELEMENT SYMBOLS, SEE MECHANICAL LEGEND.
2. FOR ELECTRICAL ELEMENT SYMBOLS, SEE ELECTRICAL LEGEND.

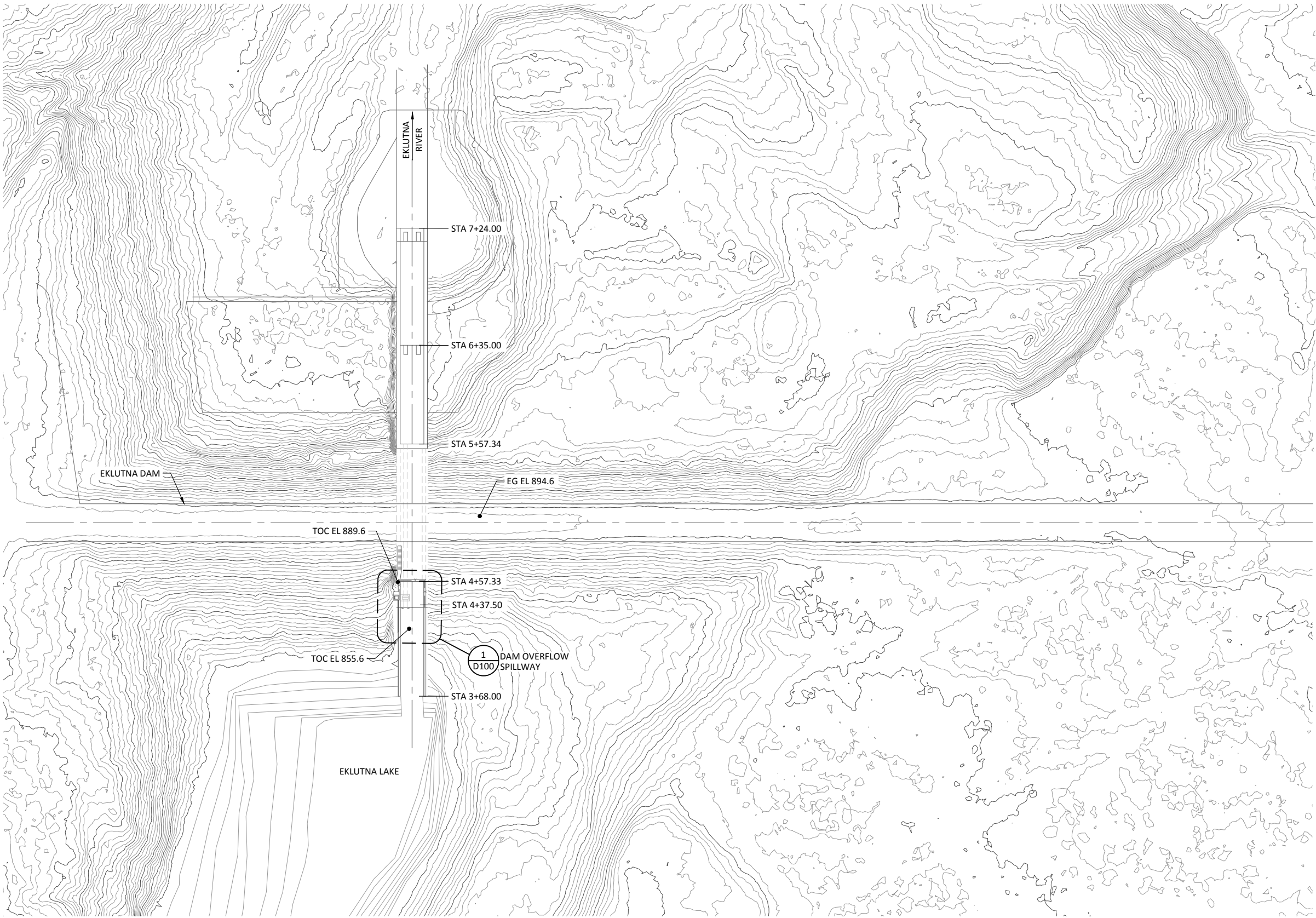
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A	04/26/24	SPE	15% DESIGN	
REV	DATE	BY	DESCRIPTION	



EKLUTNA FISH & WILDLIFE PROJECT	DESIGNED <u>S. ELLENSON</u> DRAWN <u>F. HABER</u> CHECKED <u>J. BOAG</u> PROJECT DATE <u>04/26/24</u>	DRAWING  <div style="font-size: 48pt; font-weight: bold;">G005</div>
EKLUTNA DAM SPILLWAY MODIFICATIONS		
INSTRUMENTATION AND EQUIPMENT LEGEND		





- SHEET NOTES:**
- ELEVATIONS SHOWN ARE IN NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).

**DEMOLITION KEY PLAN**

SCALE: 1"= 40'

0' 40' 80'



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**MUNICIPALITY OF ANCHORAGE**

EKLUTNA FISH & WILDLIFE PROJECT
EKLUTNA DAM SPILLWAY MODIFICATIONS
DEMOLITION KEY PLAN

DESIGNED	S. ELLENSON
DRAWN	F. HABER
CHECKED	J. BOAG
PROJECT DATE	04/26/24

DRAWING

**D001**

Path: C:\Vault\Chugach Electric\24-021 Eklutna Spillway Modification\001.dwg Plot date: Apr 23, 2024 03:42pm CAD User: Woodfon

- SHEET NOTES:
1.

ELEVATIONS SHOWN ARE IN NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
- SHEET KEY NOTES:
- A

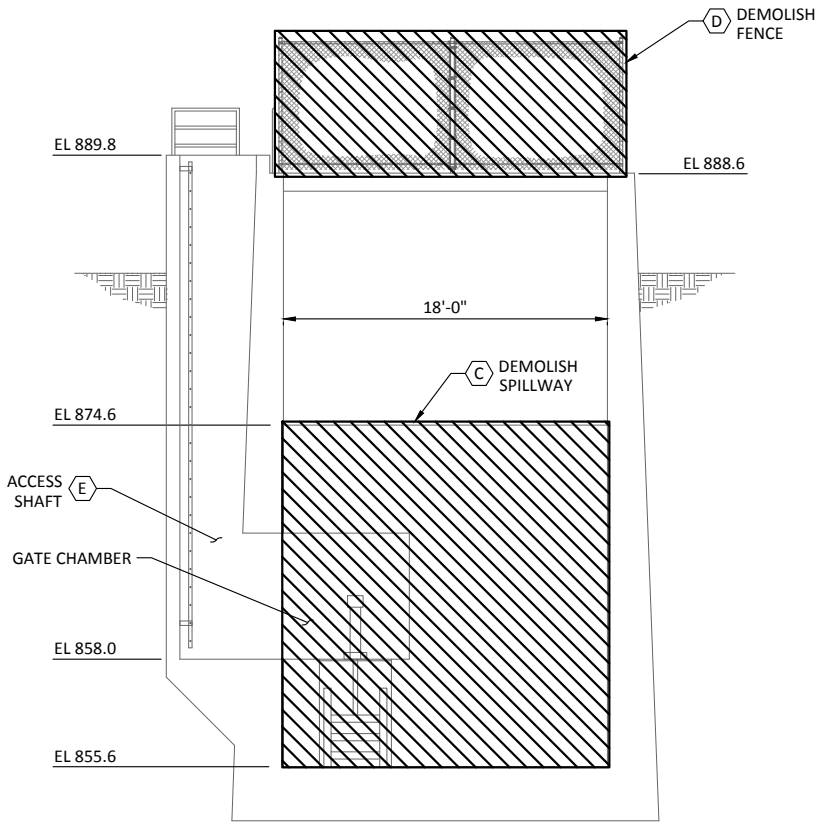
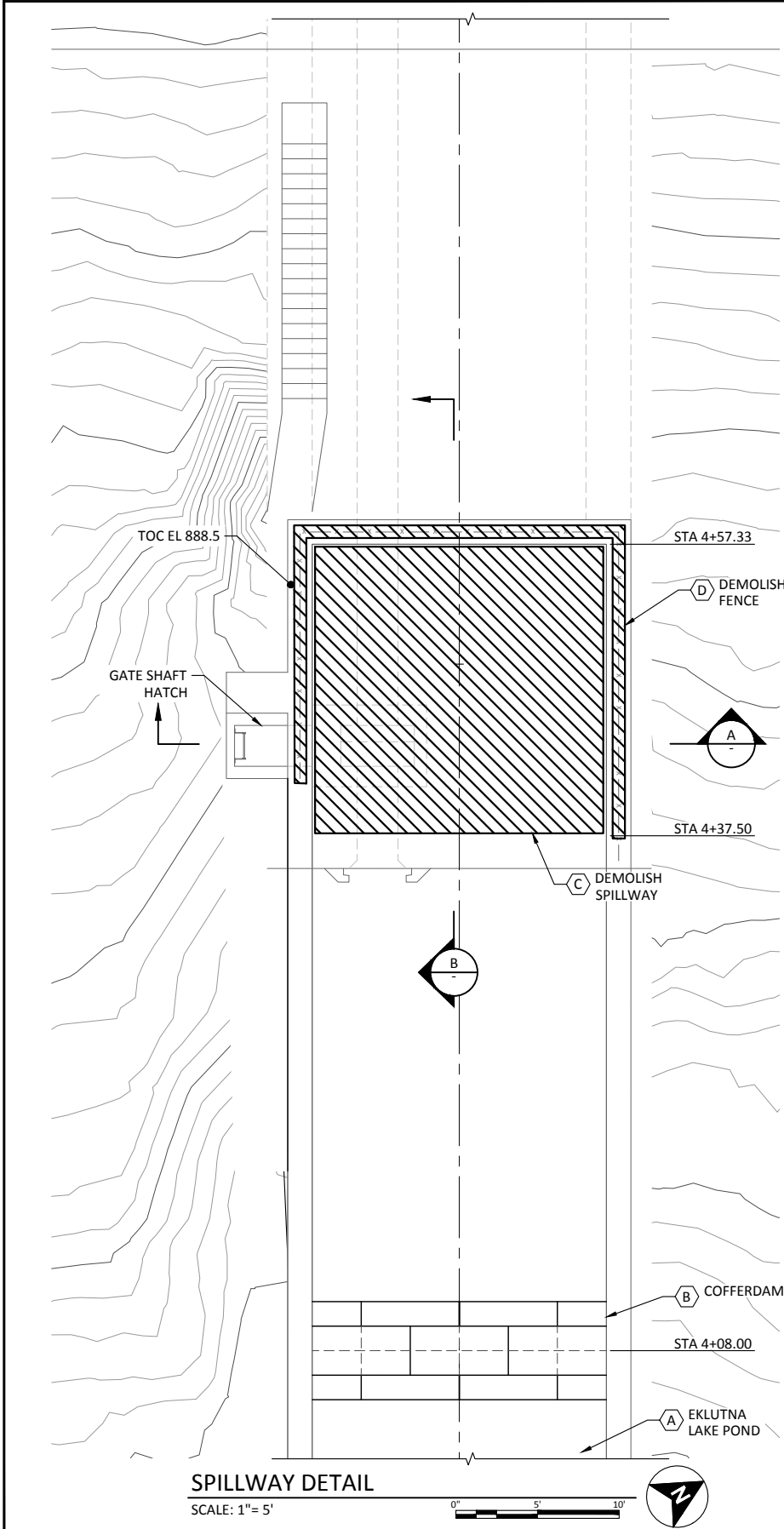
PRIOR TO BEGINNING WORK, DRAIN THE EKLUTNA LAKE POND THROUGH THE EXISTING DRAINAGE OUTLET GATE WITHIN THE DAM SPILLWAY. THIS SHALL OCCUR IN THE SPRING WHEN EKLUTNA LAKE IS DISCONNECTED FROM THE DAM.
- B

A COFFERDAM SHALL BE CONSTRUCTED AT THE INLET TO THE DAM SPILLWAY. DESIGN OF COFFERDAM AND DEWATERING SYSTEM SHALL BE DEVELOPED BY THE CONTRACTOR AND SUBMIT TO THE ENGINEER FOR APPROVAL.
- C

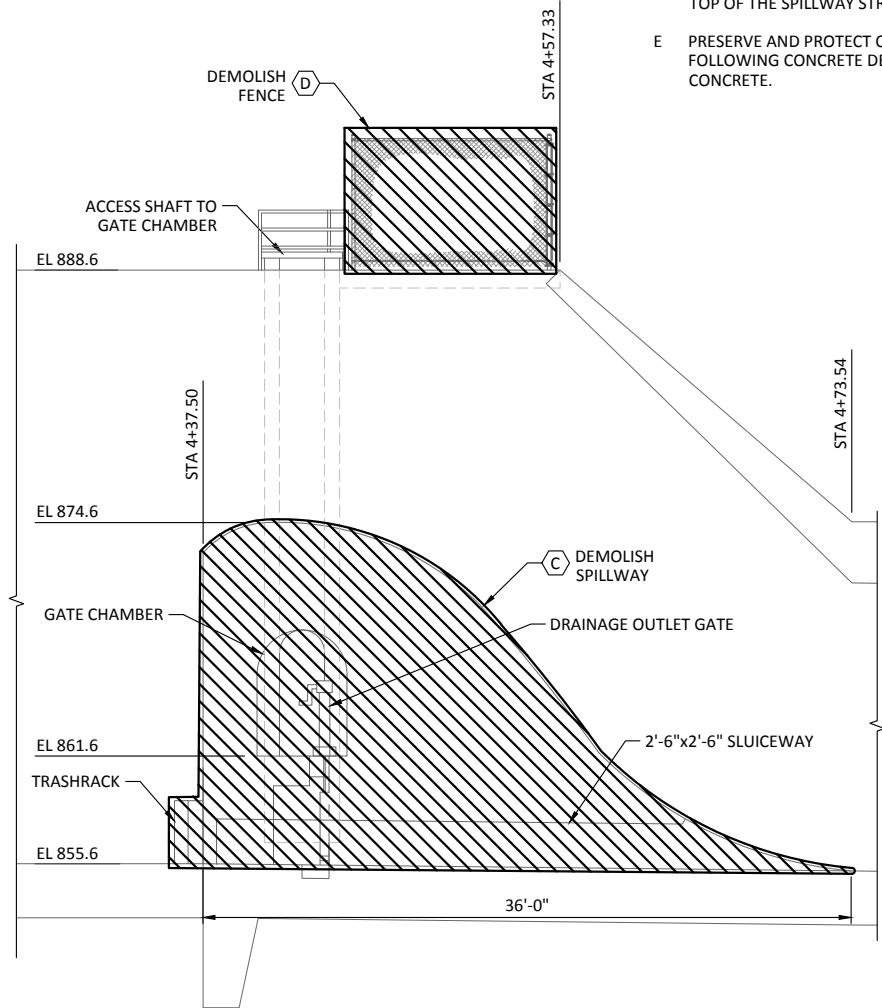
DEMOLISH AND DISPOSE OF EXISTING OGEE CREST SPILLWAY, APPROXIMATELY 235 CY OF CONCRETE.
- D

DEMOLISH AND DISPOSE OF CHAIN LINK FENCE LOCATED ON TOP OF THE SPILLWAY STRUCTURE.
- E

PRESERVE AND PROTECT OPENING TO ACCESS SHAFT FOLLOWING CONCRETE DEMOLITION. REPAIR ANY DAMAGED CONCRETE.



A SPILLWAY SECTION  
SCALE: 3/16"= 1'-0"



B SPILLWAY SECTION  
SCALE: 3/16"= 1'-0"

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REV	DATE	SPE	BY	DESCRIPTION
A	04/26/24	15% DESIGN		

WARNING

0

1/2

1

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EKLUTNA FISH & WILDLIFE PROJECT

EKLUTNA DAM SPILLWAY MODIFICATIONS

EKLUTNA DAM SPILLWAY DEMOLITION PLAN, SECTIONS, AND DETAILS

DESIGNED S. ELLENSON

DRAWN F. HABER

CHECKED J. BOAG

PROJECT DATE 04/26/24

DRAWING

D100

Path: C:\Vault\Chugach Electric\24-021 Eklutna Spillway Modification\D100.dwg Plot date: Apr 23, 2024 03:43pm, CAD User: Woodfon



- SHEET KEY NOTES:
- A

DEMOLISH AND DISPOSE OF EXISTING OGEE CREST SPILLWAY, APPROXIMATELY 235 CY OF CONCRETE.
- B

DEMOLISH AND DISPOSE OF CHAIN LINK FENCE LOCATED ON TOP OF THE SPILLWAY STRUCTURE.



1

D100

PHOTO



1

D100

PHOTO

PRELIMINARY  
NOT FOR CONSTRUCTION

A	04/26/24	SPE	15% DESIGN	
REV	DATE	BY		DESCRIPTION

WARNING

0

1/2

1

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EKLUTNA FISH & WILDLIFE PROJECT
EKLUTNA DAM SPILLWAY MODIFICATIONS
EKLUTNA DAM SPILLWAY DEMOLITION PHOTOS

DESIGNED	S. ELLENSON
DRAWN	F. HABER
CHECKED	J. BOAG
PROJECT DATE	04/26/24

DRAWING

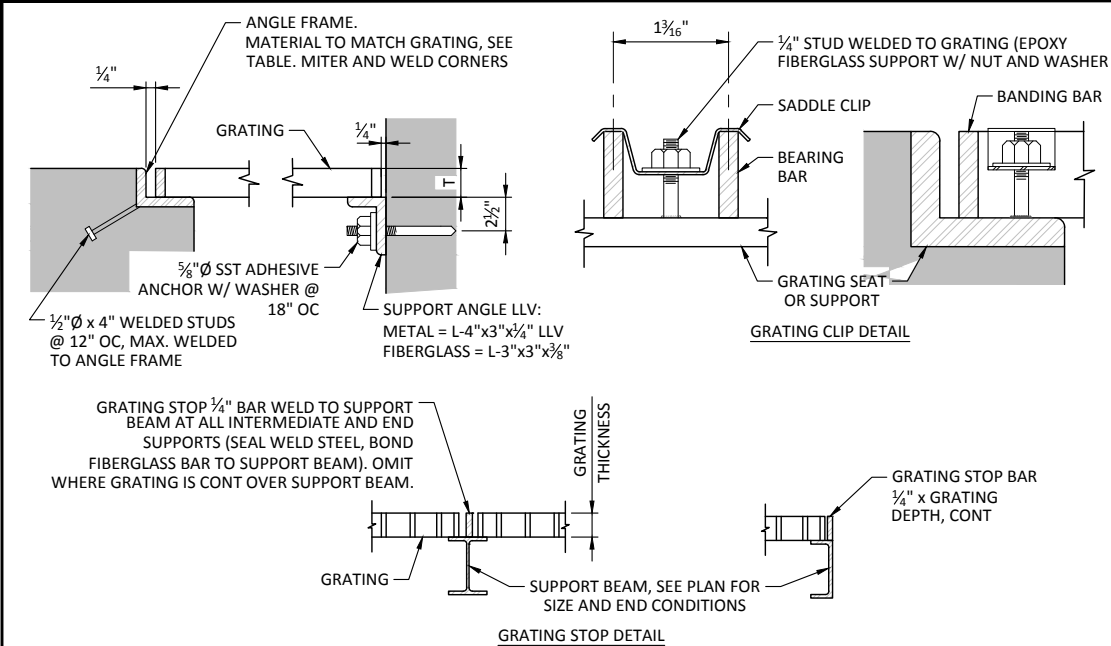
D101

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JOB NO: 000000





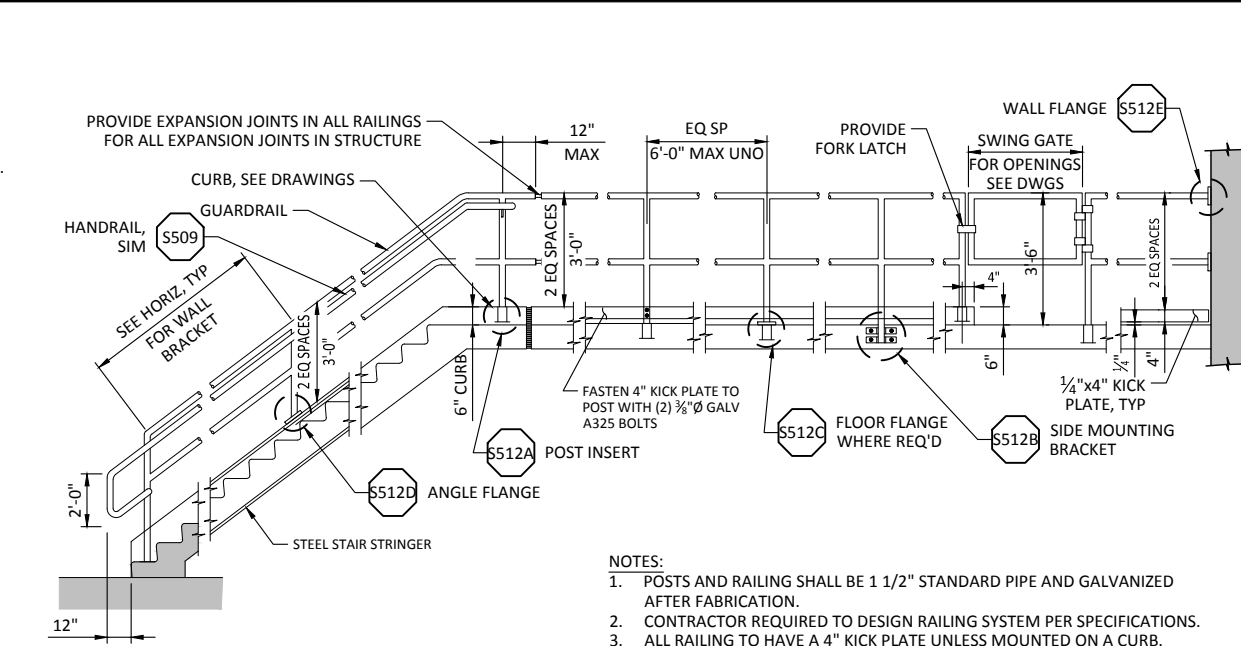


**NOTES:**

- UNLESS INDICATED OTHERWISE, ALL GRATING SHALL BE ALUM.
- GRATING DEPTH TO BE DETERMINED BY MANUFACTURER AND APPROVED BY ENGINEER, UNO.
- ALL ENDS AND OPENINGS SHALL BE Banded.
- WEIGHT OF GRATING SECTION SHALL NOT EXCEED 80 LBS.
- METAL BEARING BARS SHALL BE DEPTH T x 3/16" @ 1 3/16" OC.. CROSS BARS SHALL BE @ 4" OC.
- PROVIDE A MINIMUM OF 4 CLIPS PER GRATING PANEL, APPROX 4" FROM PANEL CORNERS. MAXIMUM CLIP SPACING AT 36" OC.
- MATERIALS:**
  - A. ALUM GRATING - USE ALUM ANGLE SUPPORTS AND SST BOLTS AND CLIPS. IF SUPPORTED BY STEEL BEAMS, USE STEEL STOP BARS. ALUM IN CONTACT WITH CONC OR STEEL SHALL BE COATED PER THE PROTECTIVE COATING SPECS.
  - B. GALV STEEL GRATING - USE STEEL ANGLE SUPPORTS, BOLTS AND CLIPS. GALV AFTER FABRICATION.
  - C. SST GRATING - USE SST ANGLE SUPPORTS, BOLTS AND CLIPS.
  - D. FIBERGLASS GRATING - USE FIBERGLASS FOR ALL COMPONENTS EXCEPT DRILLED ANCHORS; ALL CUT EDGES SHALL BE SEALED WITH RESIN; BONDING: USE EPOXY ADHESIVE BONDING AGENT

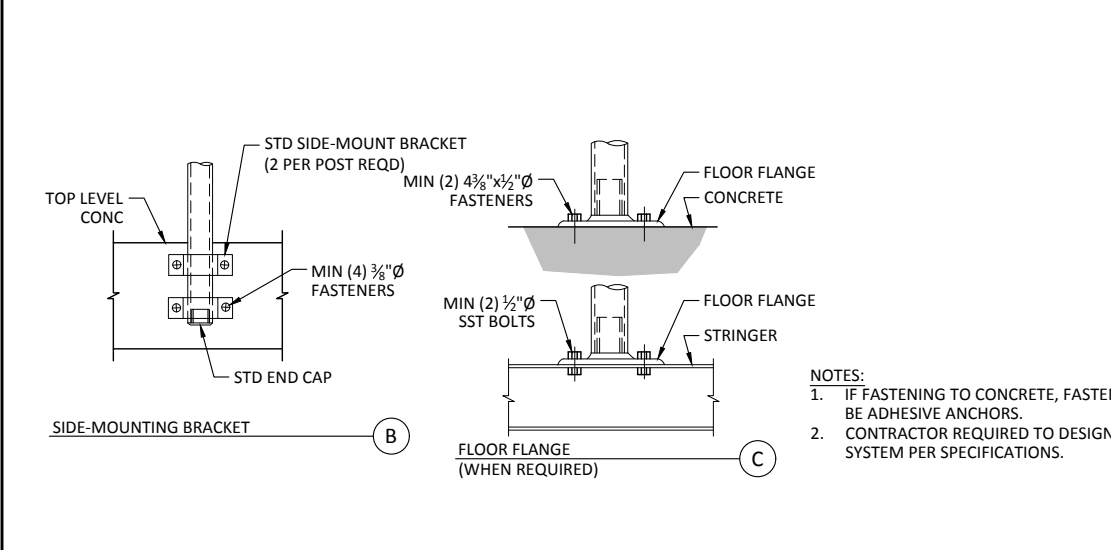
GRATING DEPTH (T)	STEEL ANGLE (STEEL)	GRATING DEPTH (T)	STEEL ANGLE (STEEL)
1"	1 3/4 x 1 3/4 x 3/4 (1 3/4 x 1 3/4 x 3/4)	2"	*2 1/2 x 2 1/2 x 1/2
1 1/4"	2 x 1 1/2 x 3/4 (1 1/2 x 1 1/2 x 3/4)	2 1/4"	2 1/2 x 2 1/2 x 3/4
1 1/2"	1 3/4 x 1 3/4 x 3/4	2 1/2"	3 x 3 x 1/2
1 3/4"	2 x 2 x 3/4		

\* OR USE 2 1/2" x 2 1/2" x 3/4" W/ 3/4" SHIM PLATE WELDED TO BOTTOM



**S501 GRATING** SCALE: NTS

**S510 TWO-RAIL RAILING GUARDRAIL DETAIL** SCALE: NTS



BEAM: ONE SIDE ONLY

BEAM: BOTH SIDES

BEAM SIZE	NO OF A325N BOLTS REQ'D		CONN PLATE THICKNESS (IN)	WELD SIZE (t)	A (IN)	B (IN)	C (IN)	D (IN)
	3/4" Ø	1" Ø						
W8	2		3/16	3/16	2 1/2"	2 1/2"	1 1/2"	1 1/2"
W10	2		3/16	3/16	3	3	1 1/2"	1 1/2"

6" OR LESS CONNECTION

NOTES:

- EXTEND CONN PLATE WHEN E1 IS GREATER THAN OR EQUAL TO W12 BM.
- LIMIT OF CONN PLATE WHEN E2-E1 IS LESS THAN OR EQUAL TO 6 INCHES.

BEAM TO CONC CONNECTION SCHEDULE

BEAM SIZE	DOUBLE ANGLE SIZE	# OF BOLTS (N)	ADHESIVE ANCHORS NUMBER AND SIZE

**S512 RAILING, GUARDRAIL AND HANDRAIL SUPPORT DETAIL** SCALE: NTS

**S562 BEAM TO BEAM CONNECTION - PLATE CONNECTION** SCALE: NTS

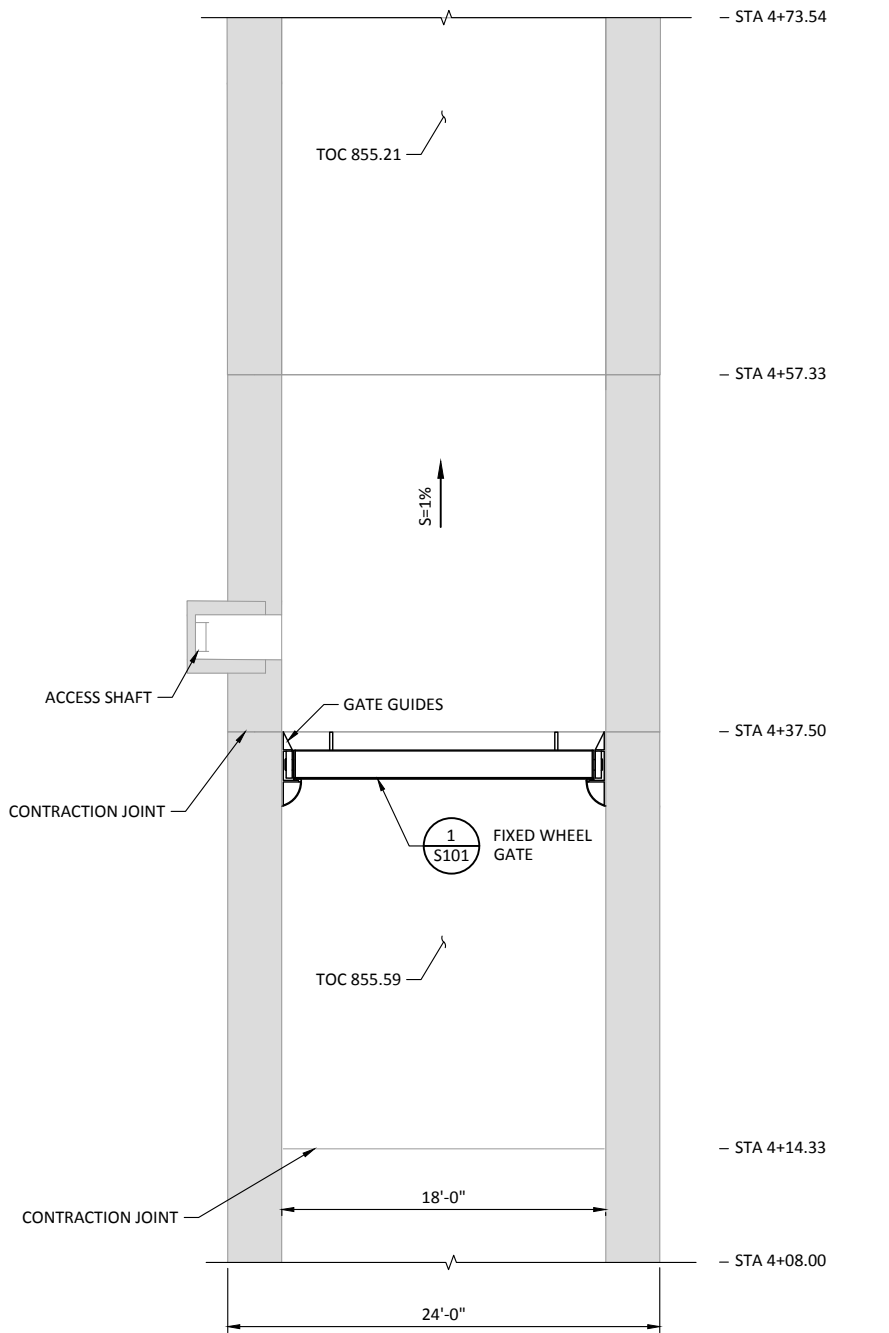
**S563 BEAM TO CONC CONNECTION** SCALE: NTS

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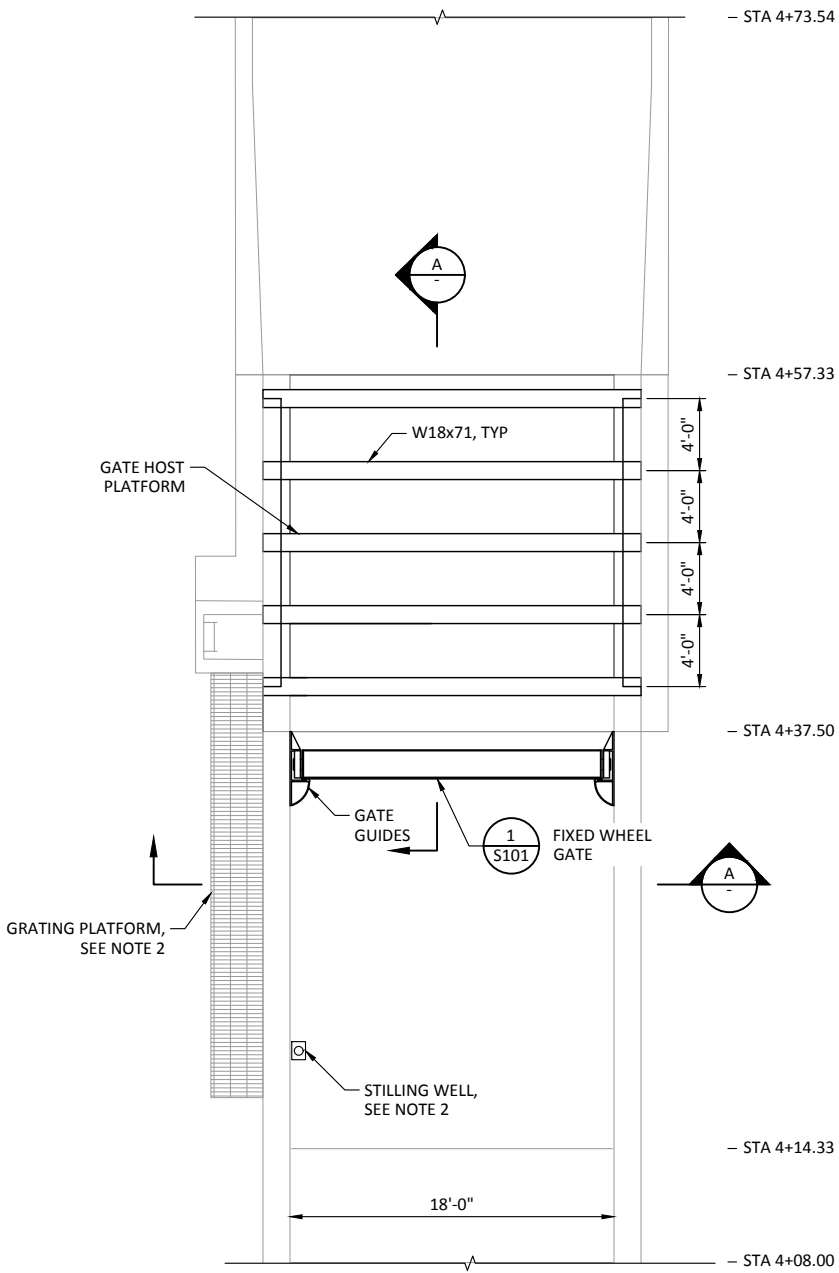
JOB NO: 000000



- SHEET NOTES:**
- 1. ELEVATIONS SHOWN ARE IN NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
  - 2. GRATING PLATFORM AND STILLING WELL INSTALLED AS PART OF THE DAM OUTLET MODIFICATION WORK PRIOR TO THE SPILLWAY MODIFICATIONS BEING INITIATED. GRATING PLATFORM AND STILLING WELL SHALL BE PRESERVED AND PROTECTED AS PART OF THIS WORK EFFORT.



**EKLUTNA DAM SPILLWAY STRUCTURAL PLAN - 862.0**  
SCALE: 3/16"= 1'-0"



**EKLUTNA DAM SPILLWAY STRUCTURAL PLAN - EL. 888.6**  
SCALE: 3/16"= 1'-0"

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REV	DATE	BY	DESCRIPTION
A	04/26/24	SPE	15% DESIGN

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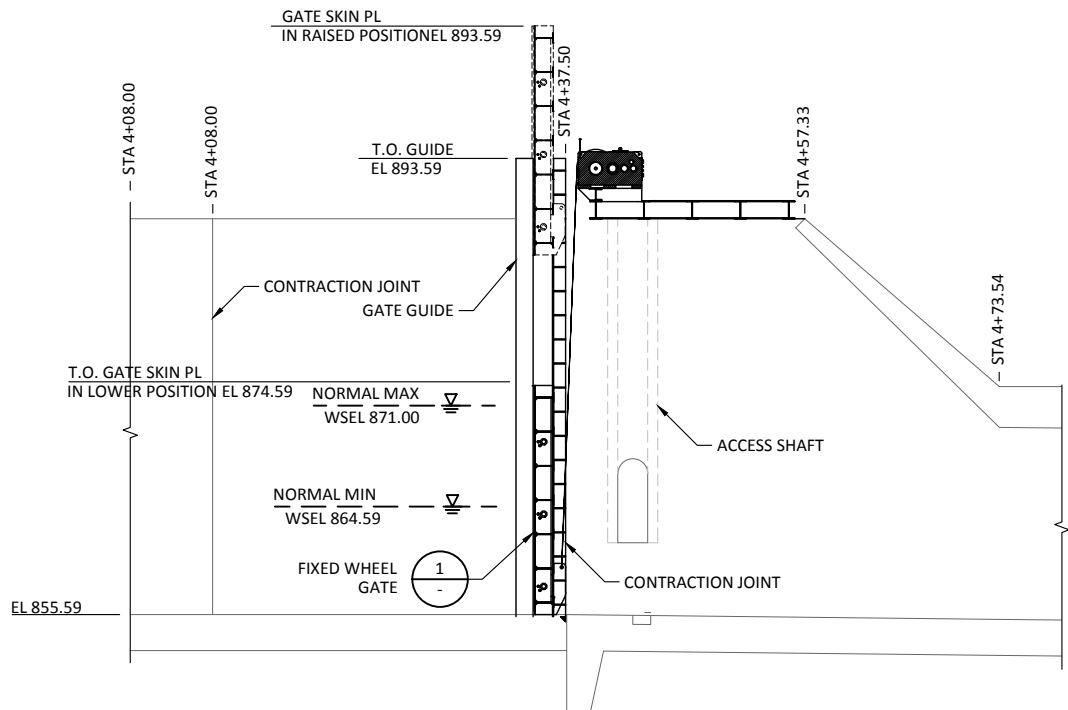
EKLUTNA FISH & WILDLIFE PROJECT
EKLUTNA DAM SPILLWAY MODIFICATIONS
EKLUTNA DAM SPILLWAY STRUCTURAL PLANS

DESIGNED	G. CLARK
DRAWN	J. HOLT
CHECKED	M. MERKLEIN
PROJECT DATE	04/26/24

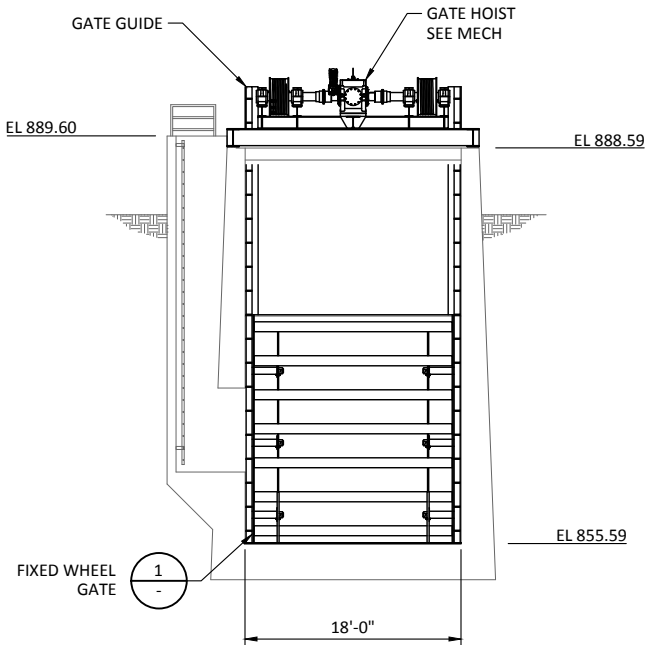
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**S100**

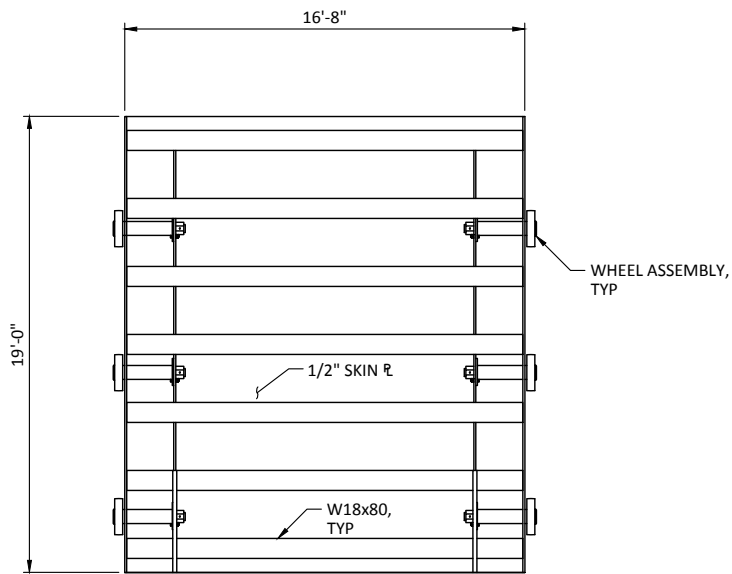
SHEET NOTE:  
1. ELEVATIONS SHOWN ARE IN NORTH AMERICAN  
VERTICAL DATUM OF 1988 (NAVD88).



A SECTION  
SCALE: 1/8" = 1'-0"



B SECTION  
SCALE: 1/8" = 1'-0"



1 DETAIL  
SCALE: 1/4" = 1'-0"

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EKLUTNA FISH & WILDLIFE PROJECT
EKLUTNA DAM SPILLWAY MODIFICATIONS
EKLUTNA DAM SPILLWAY STRUCTURAL SECTIONS

DESIGNED	G. CLARK
DRAWN	J. HOLT
CHECKED	M. MERKLEIN
PROJECT DATE	04/26/24

DRAWING

S101

Path: C:\Vault\Chugach Electric\24-021 Eklutna Spillway Modification\S101.dwg Plot date: Apr 23, 2024 03:43pm, CAD User: WoodRon

JOB NO: 000000

**DIMENSIONS AND TOLERANCES:**

1. UNLESS OTHERWISE NOTED PROVIDE THE FOLLOWING DIMENSIONAL TOLERANCES:

FRACTIONAL `1/16"  
X `0.06"  
X.X `0.06"  
X.XX `0.010"  
X.XXX `0.002"

2. FEATURES OF PARTS AND ASSEMBLIES THAT ARE IMPLIED AS PARALLEL OR PERPENDICULAR MUST BE PARALLEL OR PERPENDICULAR WITHIN ANGULAR TOLERANCE.
3. INFORMATION, TOLERANCES, AND NOTES ON THIS SHEET APPLY TO ALL MECH. DRAWINGS, UNLESS NOTED OTHERWISE.
4. DIMENSIONS AND TOLERANCE ARE PER ASME Y14.5 UNLESS INDICATED OTHERWISE.
5. CYLINDRICAL LIMITS AND FITS ARE PER ASME B4.1 UNLESS INDICATED OTHERWISE.
6. THREAD CALLOUTS ARE PER ASME Y14.6 UNLESS INDICATED OTHERWISE.
7. SURFACE TEXTURE IS PER ASME B46.1.
8. PROVIDE A MAXIMUM OF 125 MICROINCH SURFACE TEXTURE ON ALL MACHINED SURFACES, UNLESS SHOWN OTHERWISE.
9. LINEAR UNITS ARE FEET AND INCHES OR INCHES AND ALL ANGULAR UNITS ARE DEGREES, UNLESS SHOWN OTHERWISE.
10. BREAK CORNERS AND EDGES, UNLESS SHOWN OTHERWISE.

FABRICATION AND INSTALLATION:

1. FABRICATION OF EQUIPMENT SHALL BE IN GENERAL CONFORMANCE WITH THE LATEST REVISION OF AISC STEEL CONSTRUCTION MANUAL.
2. SURFACE FINISH, WHERE GIVEN, IS ACCORDING TO ANSI B46.1. SURFACES NOT MARKED SHALL HAVE A FINISH OF 250 MICRO-INCHES OR BETTER, EXCEPT FOR SURFACES IN CONTACT WITH GASKETED BEARING SURFACES SHALL BE 125 MICRO-INCHES OR BETTER. NO MACHINING IS REQUIRED IF OTHERWISE FINISHED (ROLLED, CAST, ETC) SURFACE IS BETTER THAN THE INDICATED REQUIREMENTS, EXCEPT IF INDICATED OTHERWISE. SURFACES TO BE MACHINED SHALL INCLUDE ALL SURFACES REQUIRED TO BE PRECISELY ALIGNED INCLUDING BEARING AND SURFACES, BUSHINGS, PINS AND OTHER CRITICAL BEARING SURFACES. SURFACE FINISH IN EACH CASE SHALL BE AS SHOWN ON THE SHOP DRAWINGS. IF NOT SHOWN, THE FINISH SHALL BE SUBJECT TO APPROVAL BY THE DISTRICT.
3. TOLERANCED MACHINE FINISHES TO BE FINAL, AS INSTALLED VALUES, TOLERANCES NOT BE TAKEN PRIOR TO WELDING OR PAINTING.
4. LOCTITE NICKEL ANTI-SEIZE LUBRICANT FORMULA 771 (OR APPROVED EQUAL) SHALL BE APPLIED TO THE THREADS OF ALL FASTENERS PRIOR TO ASSEMBLY TO PREVENT GALLING. TORQUE VALUES USED TO BASED ON LUBRICATED VALUES.
5. DIMENSIONS SHALL BE MEASURED CALIBRATED MEASURING DEVICES SUITABLE FOR DIMENSIONS TAKEN. CALIBRATED TAPE MEASUREMENTS SUITABLE FOR FRACTIONAL DIMENSIONS ONLY. MICROMETERS OR CALIPERS WITH RESOLUTION TO SUITABLE FOR THE TOLERANCE OF THE PART TO BE USED. MAKE CALIBRATION RECORDS AVAILABLE AS REQUESTED BY ENGINEER.
6. LIKE PARTS SHALL BE INTERCHANGEABLE.
7. ROUNDING OF EDGES WHEN SHOWN SHALL BE FULL RADIUS.
8. PROVIDE STAINLESS STEEL SHIMS MEETING ASTM A240 TYPE 304 FOR ALIGNMENT OF MACHINERY. DEBURR ALL SHIMS PRIOR TO INSTALLATION.

FASTENERS:

1. UNLESS NOTED OTHERWISE, MAKE MECHANICAL CONNECTIONS WITH ASTM A449 HEAVY HEX BOLTS, ASTM A563 NUTS, AND ASTM F436 WASHERS. HOT DIP GALVANIZE CARBON STEEL BOLTS, NUTS, AND WASHERS UNLESS OTHERWISE NOTED.
2. USE FASTENERS THAT ARE IN ACCORDANCE WITH THE FOLLOWING DIMENSIONAL STANDARDS:
  - A. HEAVY HEX BOLTS PER ASME B18.2.6.
  - B. HEX NUTS PER ASME B18.2.6.
  - C. PLAIN WASHERS PER ASTM F436.
3. TENSION FASTENERS IN ACCORDANCE WITH THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS (RSCS) METHOD FOR TURN-OF-NUT PRETENSIONING. ROTATE NUT AFTER ACHIEVING A SNUG TIGHT CONDITION IN ACCORDANCE WITH RSCS TABLE 8.1
4. HOT DIP GALVANIZE FASTENERS PER ASTM A153. OVERSIZE NUT THREADS PER ASTM A563.

KEYWAYS:

1. MACHINE KEYS AND KEYWAYS PER ANSI B17.1 1967 (R 2017) INCLUDING THE RECOMMENDED CHAMFERS AND FILLETS PROVIDED IN THE STANDARD. SUPPLY KEYS FOR ALL ASSEMBLIES WHERE DIRECTLY INDICATED OR WHERE KEYWAYS ARE SHOWN FOR ONE OR BOTH MATING COMPONENTS.

**WELDING:**

1. PERFORM WELDING IN ACCORDANCE WITH AWS D1.1. WELDERS AND WELDING PROCEDURES SHALL BE IN ACCORDANCE WITH AWS D1.1. WELDING ELECTRODES SHALL CONFORM TO THE APPROPRIATE AWS SPECIFICATION FOR THE BASE METAL AND WELDING PROCESS USED.
2. PROVIDE WELD ACCESS HOLES TO AVOID INTERSECTING WELDS PER AISC STEEL CONSTRUCTION MANUAL 15TH ED. FIGURE C-J.1. 2 ALTERNATIVE 1 AND GRIND SMOOTH.
3. GRIND, BLEND, AND DRESS WELDS WITH SURFACE IRREGULARITIES AND SHARP CORNERS TO AVOID STRESS RISERS AND FOR PROPER ADHESION OF THE COATING BEFORE PROCEEDING WITH THE OVERALL SURFACE PREPARATION FOR COATING. REMOVE WELD SPATTER TO ENSURE PROPER ADHESION OF THE COATING. AREAS OF HIGH STRESS AND MACHINED SURFACES SHALL BE PROTECTED FROM WELD SPATTER.
4. CLEAN AND CAREFULLY EXAMINE COMPLETED WELDS FOR INSUFFICIENT THROAT OR LEG SIZES, CRACKS, UNDERCUTTING, OVERLAP, EXCESSIVE CONVEXITY OR REINFORCEMENT AND OTHER SURFACE DEFECTS TO ENSURE COMPLIANCE WITH THE REQUIREMENTS OF AWS D1.1. PERFORM NON-DESTRUCTIVE EXAMINATION BY ULTRASONIC METHOD FOR BUTT WELDS. EXAMINE OTHER WELDS BY DYE PENETRANT (PT) METHOD; MAGNETIC PARTICLE (MT); OR ULTRASONIC TESTING (UT).
5. PERFORM RANDOM SPOT CHECK EXAMINATION OF WELDS AS REQUESTED BY THE ENGINEER AS PART OF THE EQUIPMENT EXAMINATION.
6. PREHEAT THE BASE METAL AS NECESSARY TO MINIMIZE DISTORTION BEFORE WELDING. USE WELD PROCEDURES THAT CONTROL HEAT TO AVOID ADVERSELY AFFECT THE MECHANICAL PROPERTIES OF THE BASE METAL. AFTER WELDING IS COMPLETE, STRESS RELIEVE WELDMENTS BY HEAT TREATMENT IN ACCORDANCE WITH AWS D1.1. USE PROPER RESTRAINT AND SUPPORT DURING HEAT TREATMENT TO MINIMIZE DISTORTION.

### COMPONENT NAMING:


1. HOIST COMPONENT NAMING HAS BEEN PROVIDED WITH THE FOLLOWING NAMING CONVENTION:

[GATE]-[MECH COMPONENT ABBR]-[COMPONENT NUMBER]

MECHANICAL COMPONENT ABBREVIATIONS	
ABBR	COMPONENT TYPE
B	BRAKE
D	DRUM
EN	ENCODER
GC	GATE CONNECTION
LP	LOAD PIN
LS	LIMIT SWITCH
M	MOTOR
MC	MACHINERY COVER
MS	MACINERY SKID
R	REDUCER
S	SHAFT
SB	SHAFT BEARING
SC	SHAFT COUPLING
SH	SHEAVE
W	WIRE ROPE

A	04/26/24	SPE	15% DESIGN
REV	DATE	BY	DESCRIPTION

**WARNING**



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DRAWING IS NOT TO SCALE



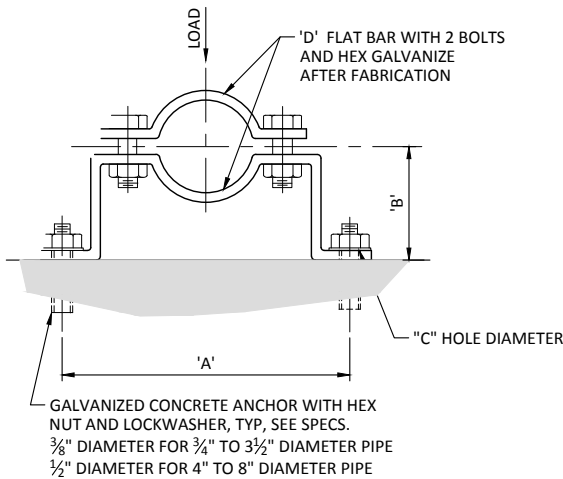
EKLUTNA FISH & WILDLIFE PROJECT
EKLUTNA DAM SPILLWAY MODIFICATIONS
MECHANICAL GENERAL NOTES

PRELIMINARY  
NOT FOR CONSTRUCTION

DESIGNED A. DESANTIS  
DRAWN J. HOLT  
CHECKED S. ELLENSON  
PROJECT DATE 04/26/24

## DRAWING

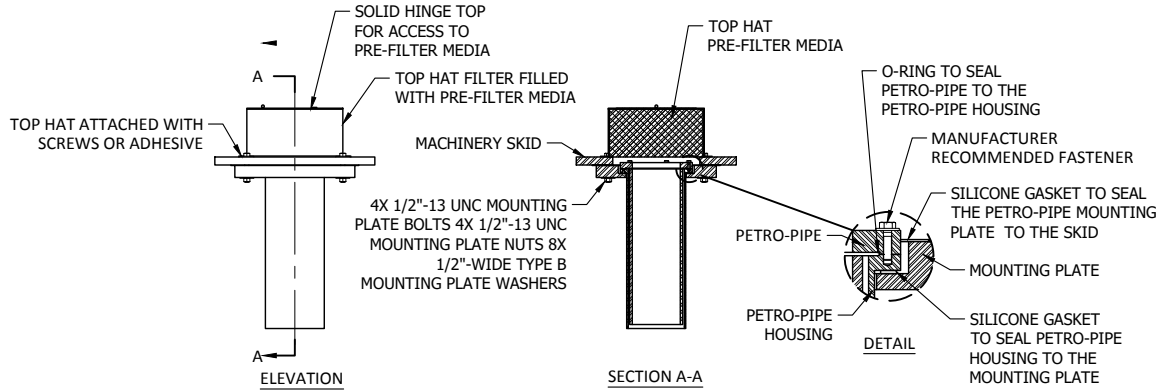
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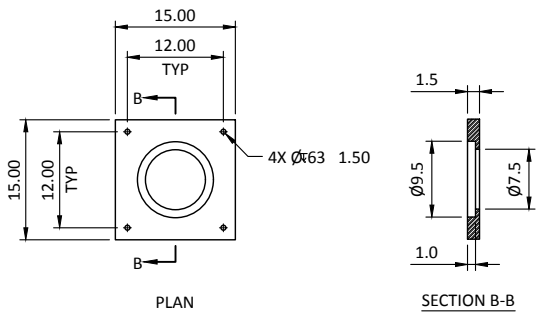
DIMENSIONS IN INCHES					LOAD RATING LBS
PIPE DIAMETER	"A"	"B" SEE NOTE 3 BELOW	"C" HOLE DIAMETER	"D" FLAT BAR SIZE	
3/4	5 15/16	2 1/2	7/16	3/16 x 1 1/4	190
1	6 1/4	2 5/8	7/16	3/16 x 1 1/4	190
1 1/4	6 11/16	2 3/4	7/16	3/16 x 1 1/4	190
1 1/2	6 15/16	3	7/16	3/16 x 1 1/4	190
2	8 5/16	3 3/16	7/16	3/4 x 1 1/4	420
2 1/2	8 7/8	3 7/16	7/16	3/4 x 1 1/4	420
3	9 1/8	3 3/4	7/16	3/4 x 1 1/4	420
3 1/2	10 1/16	4	7/16	3/4 x 1 1/4	420
4	10 9/16	4 1/4	9/16	3/4 x 1 1/2	610
5	11 3/4	4 3/4	9/16	3/4 x 1 1/2	610
6	14 3/8	5 1/16	9/16	3/8 x 1 1/2	870
8	16 5/8	6 1/16	9/16	3/8 x 1 1/2	870

- NOTE:
- WHERE SUBMERGED, PIPE CLAMP, ANCHORS, SHIELD, NUTS AND LOCKWASHER TO BE TYPE 316 STAINLESS STEEL.
  - WHEN USED WITH PVC OR FIBERGLASS PIPE, PROVIDE STEEL SHIELD WITH LOOSE FIT AROUND PIPE AT CLAMP. WRAP COPPER TUBING WITH 2" WIDE STRIP OF RUBBER AT CLAMP.
  - FOR FLANGED PIPING, INCREASE 'B' DIMENSION AS REQUIRED.
  - FOR PIPES SMALLER THAN 3/4" SEE STANDARD DETAIL M142.

M117 PIPE CLAMP FOR INDIVIDUAL PIPES  
SCALE: NTS



PETRO-PIPE ASSEMBLY



- NOTES:
- MOUNTING PLATE BOLT LENGTHS WILL DEPEND ON THE THICKNESS OF THE SKID BASE PLATE.
  - MATCH DRILL MOUNTING PLATE BOLT HOLES IN MACHINERY SKID BASE PLATE.

PETRO-PIPE MOUNTING PLATE

M750 PETRO-PIPE  
SCALE: NTS

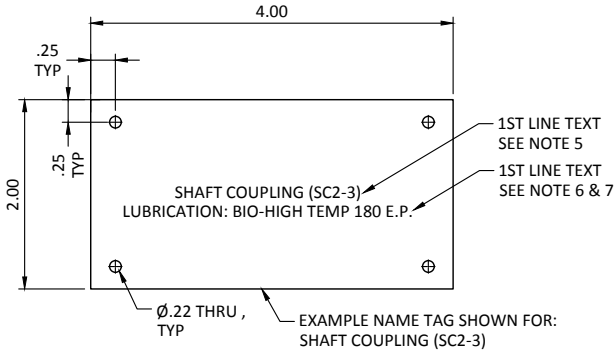


TABLE 1 -- NAMEPLATE INFO			
COMPONENT NAME	PART. NO.	LUBRICATION	TOP PLY COLOR
MOTOR	M	NA	BLACK
BRAKE	B1, B2	NA	BLACK
DRUM	D1, D2	NA	BLACK
REDUCER	R1, R2, R3	PANOLINE MARGEAR GEAR OIL	BLUE
SHAFT	S1	NA	BLACK
SHAFT BEARING	SB1-1, SB1-2, SB1-3, SB1-4, SB1-5, SB1-6, SB1-7, SB1-8, SB2-1, SB2-2	BOI-HIGH TEMP 180 EP	GREEN
SHAFT COUPLING	SC1, SC2-1, SC2-2, SC2-3, SC2-4, SC3-1, SC3-2	BOI-HIGH TEMP 180 EP	GREEN
LIMIT SWITCH	LS1, LS2, LS3	NA	BLACK
ENCODER	EN	NA	BLACK
MACHINERY SKID	MS1, MS2-1, MS2-2, MS3, MS4	NA	BLACK

- NOTES:
- PROVIDE POLYMER NAMEPLATES FOR EQUIPMENT AS INDICATED
  - MFABRICATE NAMEPLATES FROM IMPACT RESISTANT, UV STABILIZED, 2-PLY, ACRYLIC LAMACOID RATED FOR OUTDOOR USE.
  - THE TOP LAMACOID PLY SHALL BE THE COLOR INDICATED. THE BOTTOM LAMACOID PLY COLOR SHALL BE WHITE.
  - ENGRAVE LETTERING THROUGH THE TOP LAMACOID PLY
  - 1ST LINE TEXT SHALL BE: COMPONENT NAME PER TABLE 1 (PART NO. PER TABLE 1).
  - 2ND LINE TEXT SHALL BE: "LUBRICATION:" (LUBRICATION PER TABLE 1).
  - IF COMPONENT IS NOT LUBRICATED (NA IN TABLE 1) THERE IS NO 2ND LINE TEXT.
  - PROVIDE NAMEPLATES WITH MOUNTING HOLES IN CORNERS FOR SOLID RIVETS OR SCREWS.

M605 MACHINERY NAMEPLATE  
SCALE: NTS

PRELIMINARY  
NOT FOR CONSTRUCTION

REV	DATE	SPE BY	15% DESIGN	DESCRIPTION
A	04/26/24			

WARNING

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MEA

MATANUSKA ELECTRIC ASSOCIATION

MUNICIPALITY OF ANCHORAGE

EKLUTNA FISH & WILDLIFE PROJECT
EKLUTNA DAM SPILLWAY MODIFICATIONS
MECHANICAL STANDARD DETAILS

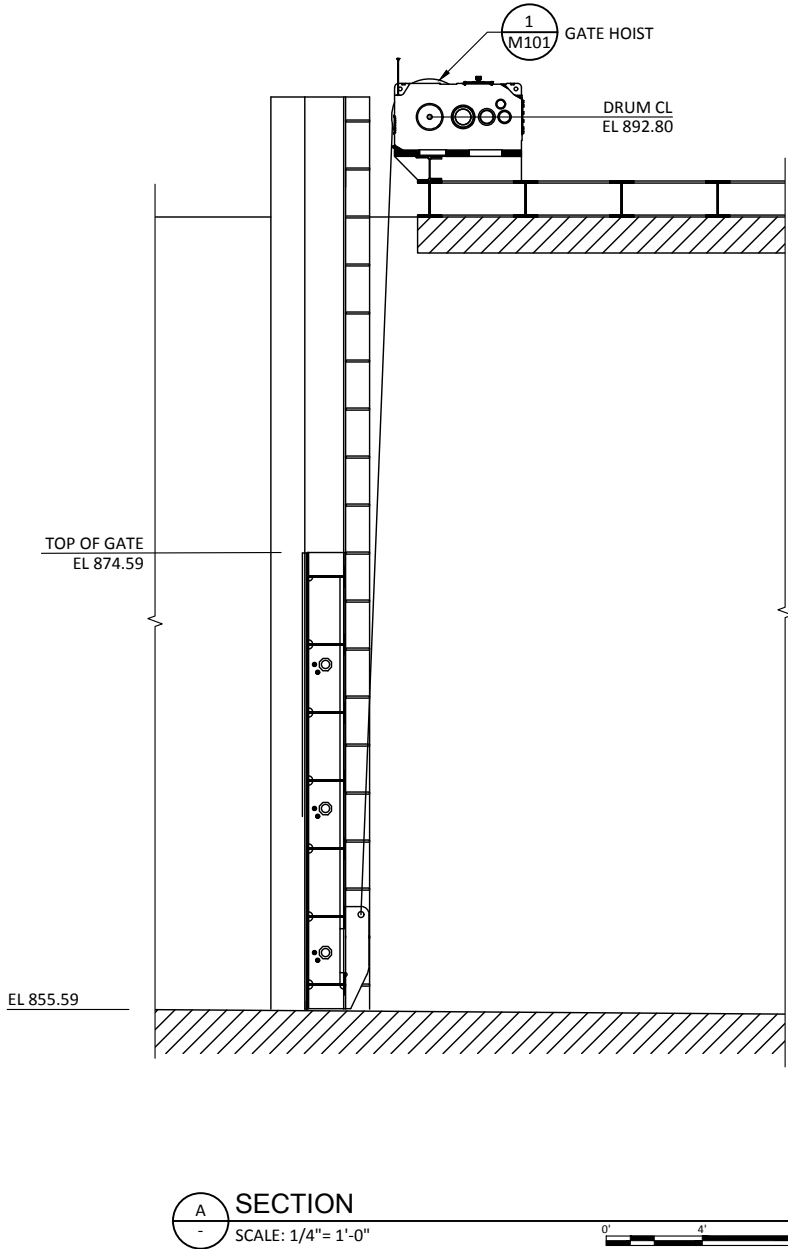
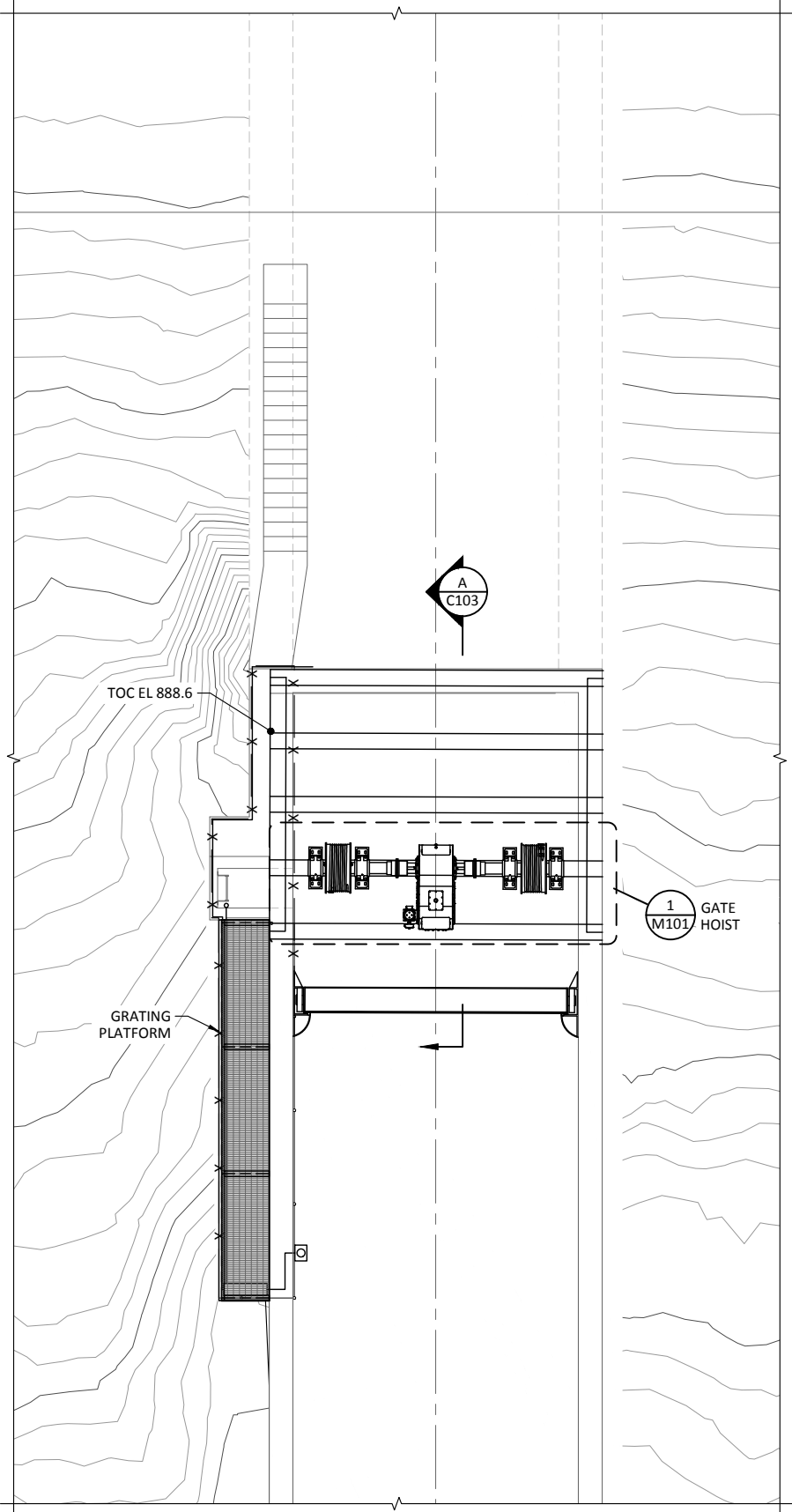
DESIGNED	A. DESANTIS
DRAWN	J. HOLT
CHECKED	S. ELLENSON
PROJECT DATE	04/26/24

DRAWING

GM002

SHEET NOTES:

1. ELEVATIONS SHOWN ARE IN NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).



EKLUTNA DAM SPILLWAY MECHANICAL PLAN - EL 888.6  
SCALE: 3/16"= 1'-0"

PRELIMINARY  
NOT FOR CONSTRUCTION

A	04/26/24	SPE	15% DESIGN	
REV	DATE	BY		DESCRIPTION

WARNING

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**MUNICIPALITY OF ANCHORAGE**

EKLUTNA FISH & WILDLIFE PROJECT

EKLUTNA DAM SPILLWAY MODIFICATIONS

EKLUTNA DAM SPILWAY MECHANICAL PLAN AND SECTIONS

DESIGNED A. DESANTIS

DRAWN J. HOLT

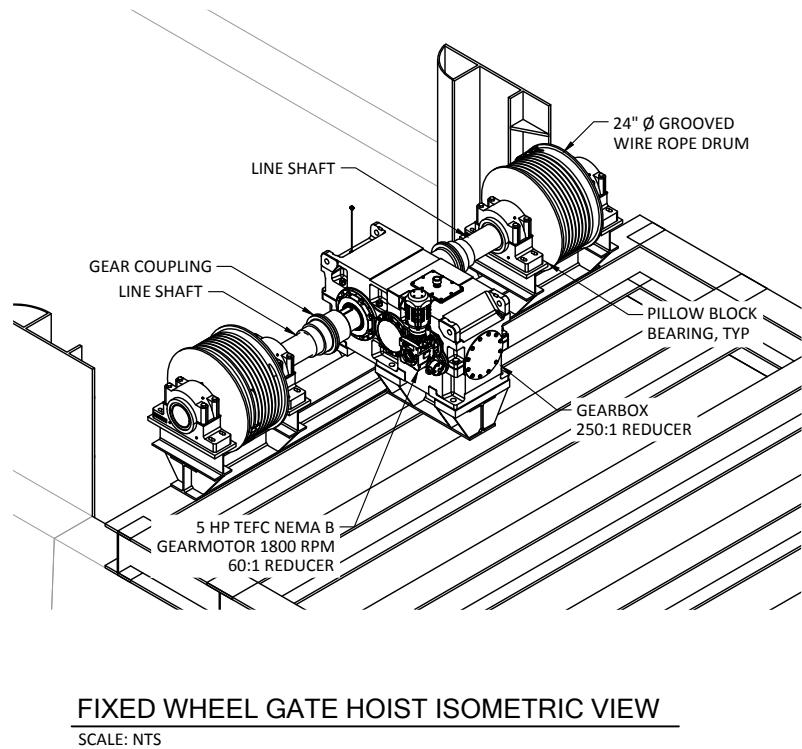
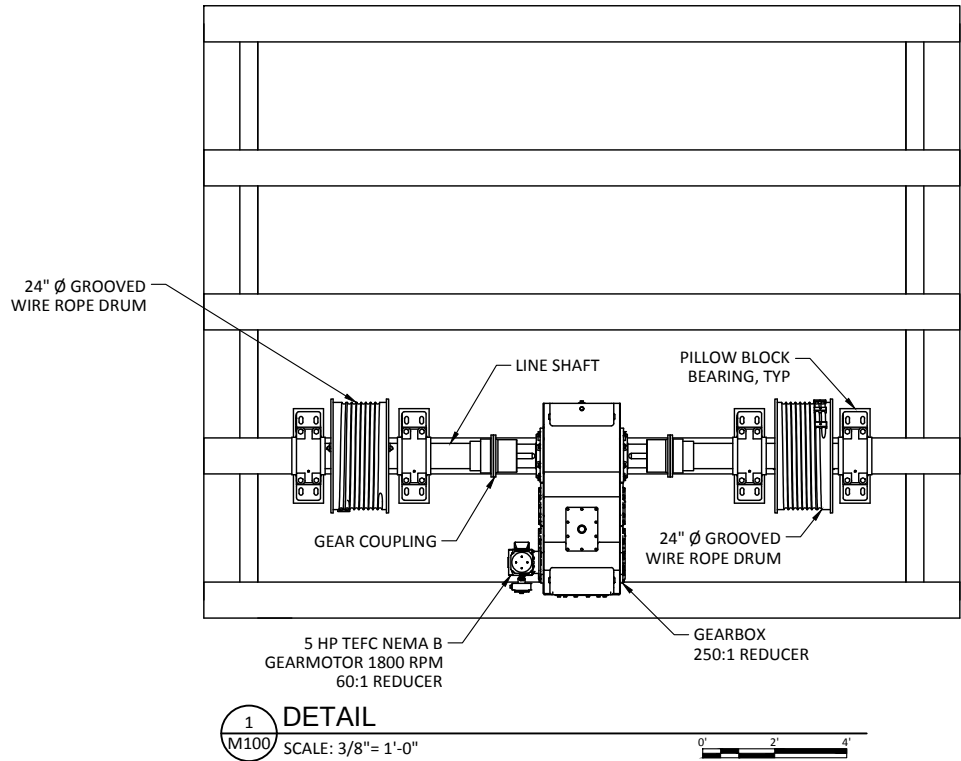
CHECKED S. ELLENSON

PROJECT DATE 04/26/24

DRAWING

**M100**

Path: C:\Vault\Chugach Electric\24-021 Eklutna Spillway Modification\M100.dwg Plot date: Apr 23, 2024 03:44pm, CAD User: WoodRon



PRELIMINARY  
NOT FOR CONSTRUCTION

REV	DATE	BY	DESCRIPTION
A	04/26/24	SPE	15% DESIGN

**WARNING**

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MUNICIPALITY OF ANCHORAGE

EKLUTNA FISH & WILDLIFE PROJECT
EKLUTNA DAM SPILLWAY MODIFICATIONS
EKLUTNA DAM SPILLWAY MECHANICAL DETAILS

DESIGNED	A. DESANTIS
DRAWN	J. HOLT
CHECKED	S. ELLENSON
PROJECT DATE	04/26/24

DRAWING

**M101**

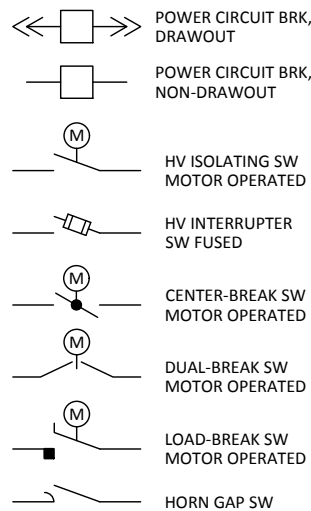
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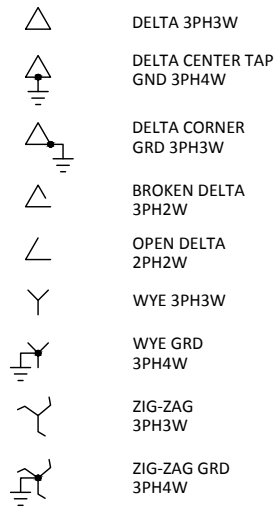


## DIAGRAMS

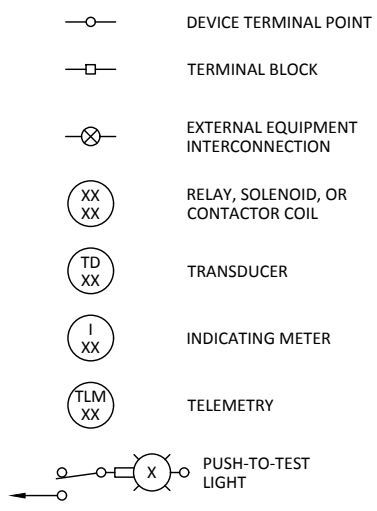
## HIGH - MEDIUM VOLTAGE SWITCHING



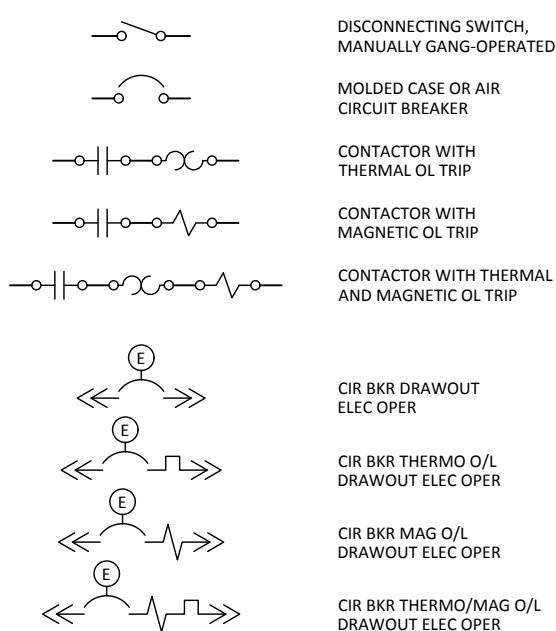
### TRANSFORMERS WINDING CONNECTIONS:



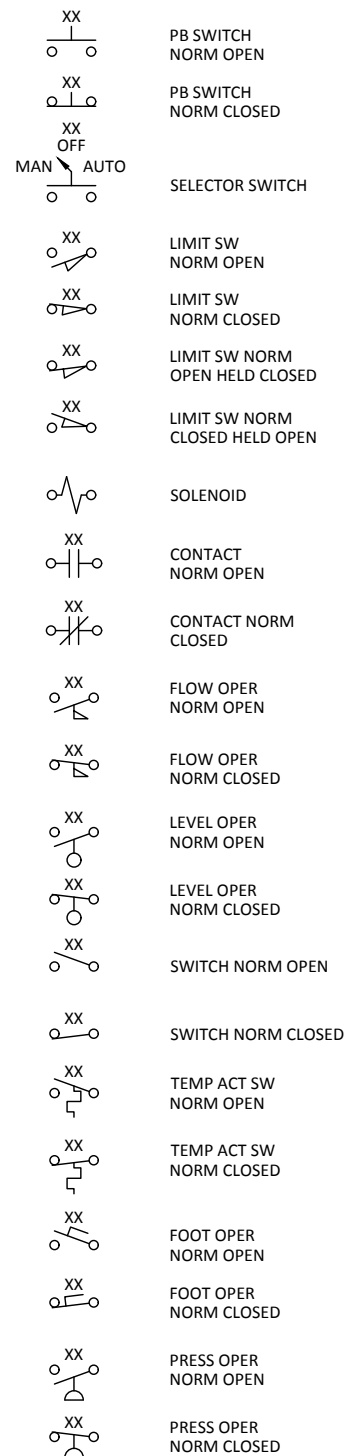
### MISC DEVICES & CONNECTIONS:



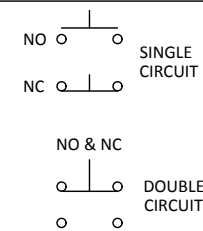
### LOW VOLTAGE SWITCHING:



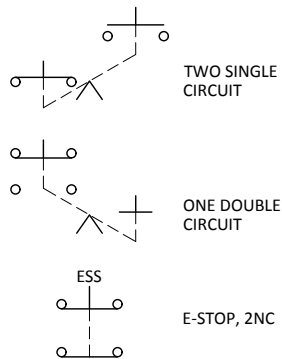
### CONTROL SWITCHING:



MOMENTARY CONTACTS:



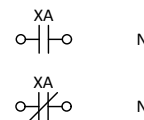
MAINTAINED CONTACTS:



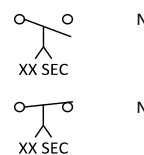
## INSTANT OPERATION CONTACTS WITH BLOWOUT



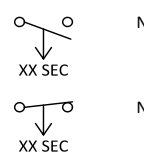
INSTANT OPERATION  
CONTACTS WITHOUT BLOWOUT



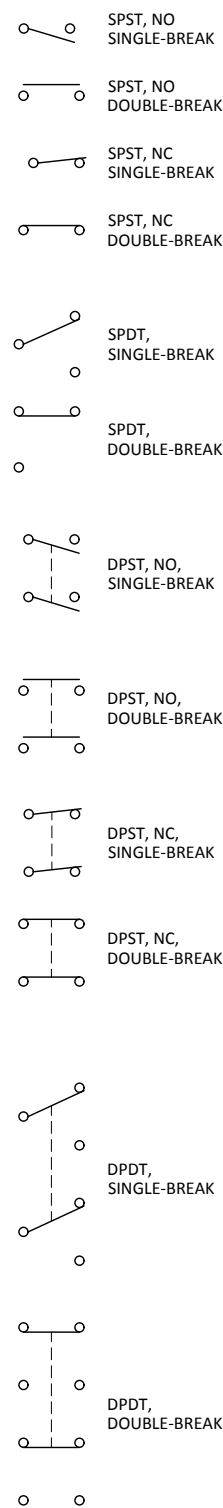
### TIMED CONTACTS - CONTACT ACTION DELAYED AFTER COIL IS ENERGIZED



### TIMED CONTACTS - CONTACT ACTION DELAYED AFTER COIL IS DE-ENERGIZED



SUPPLEMENTARY  
CONTACTS SYMBOLS:



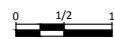
**NOTE:**

1. "X" OR "XX" SHOWN ON SYMBOLS WILL BE SUBSTITUTED WITH DEVICE FUNCTION NUMBERS, LETTER SUFFIXES, PILOT LIGHT COLORS, OR OTHER DESCRIPTIVE TEXT, WHICH ARE DEFINED ELSEWHERE IN THESE LEGEND DRAWINGS.

PRELIMINARY  
NOT FOR CONSTRUCTION

A	04/26/24	SPE	15% DESIGN
REV	DATE	BY	DESCRIPTION

**WARNING**



IF THIS BAR DOES NOT  
MEASURE 1" THEN  
DRAWING IS NOT TO SCALE



EKLUTNA FISH &amp; WILDLIFE PROJECT

EKLUTNA DAM SPILLWAY MODIFICATIONS

ELECTRICAL STANDARD SYMBOLS 1

DESIGNED C. CURTIS

DRAWN F. HABER

CHECKED J. BAKKEN

PROJECT DATE 04/26/24

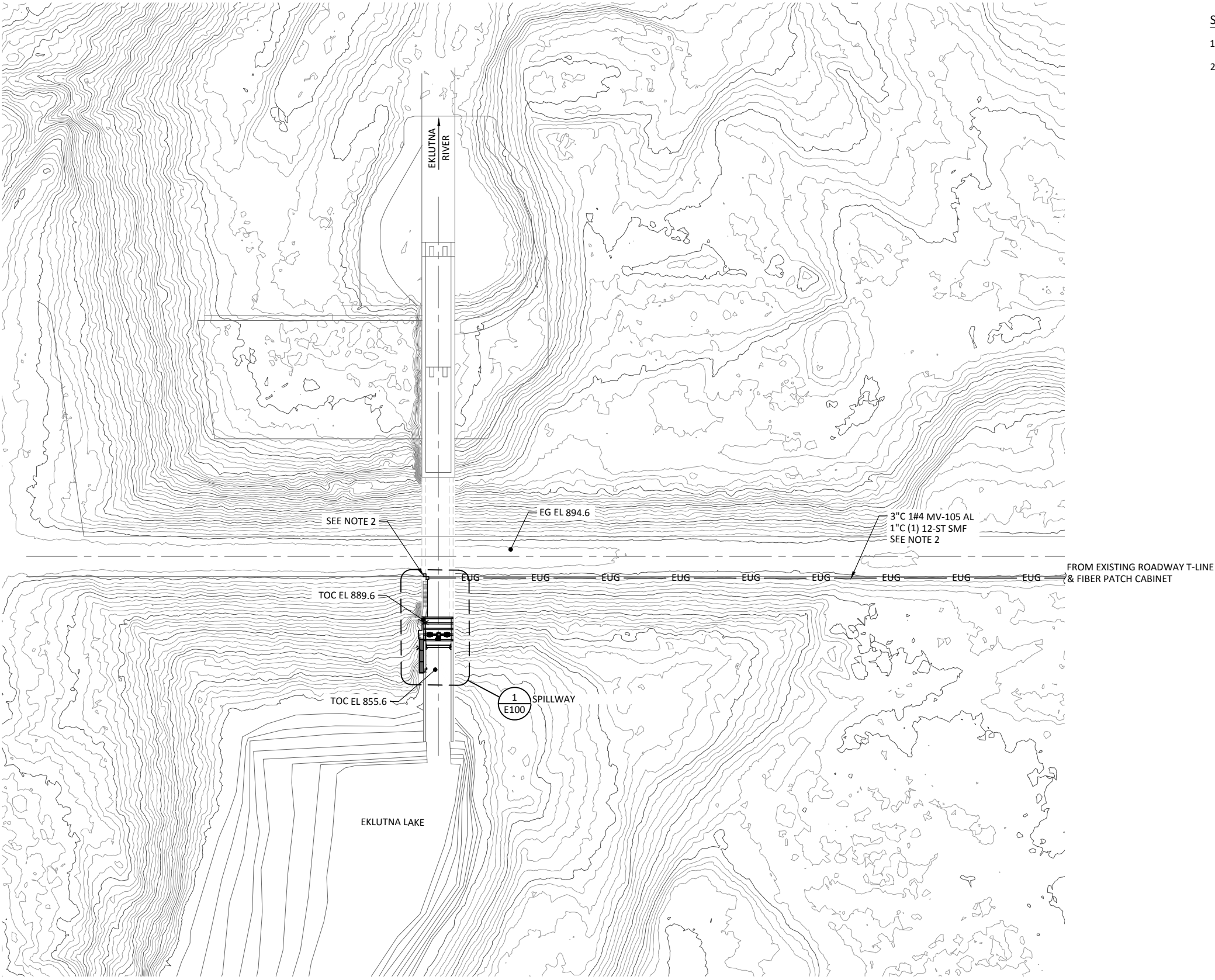
DRAWING

# GE002





- SHEET NOTES:
- 1. ELEVATIONS SHOWN ARE IN NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
  - 2. BURIED POWER LINE, FIBER OPTIC COMMUNICATION LINES, AND CONCRETE HANHOLES INSTALLED PREVIOUSLY DURING DAM OUTLET GATE MODIFICATION WORK.



INSTALLATION KEY PLAN  
SCALE: 1"= 40'

PRELIMINARY  
NOT FOR CONSTRUCTION

A	04/26/24	SPE	15% DESIGN	
REV	DATE	BY		DESCRIPTION

WARNING

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EKLUTNA FISH & WILDLIFE PROJECT

EKLUTNA DAM SPILLWAY MODIFICATIONS

ELECTRICAL TRANSMISSION & COMMUNICATIONS UPGRADES PLAN

DESIGNED S. ELLENSON

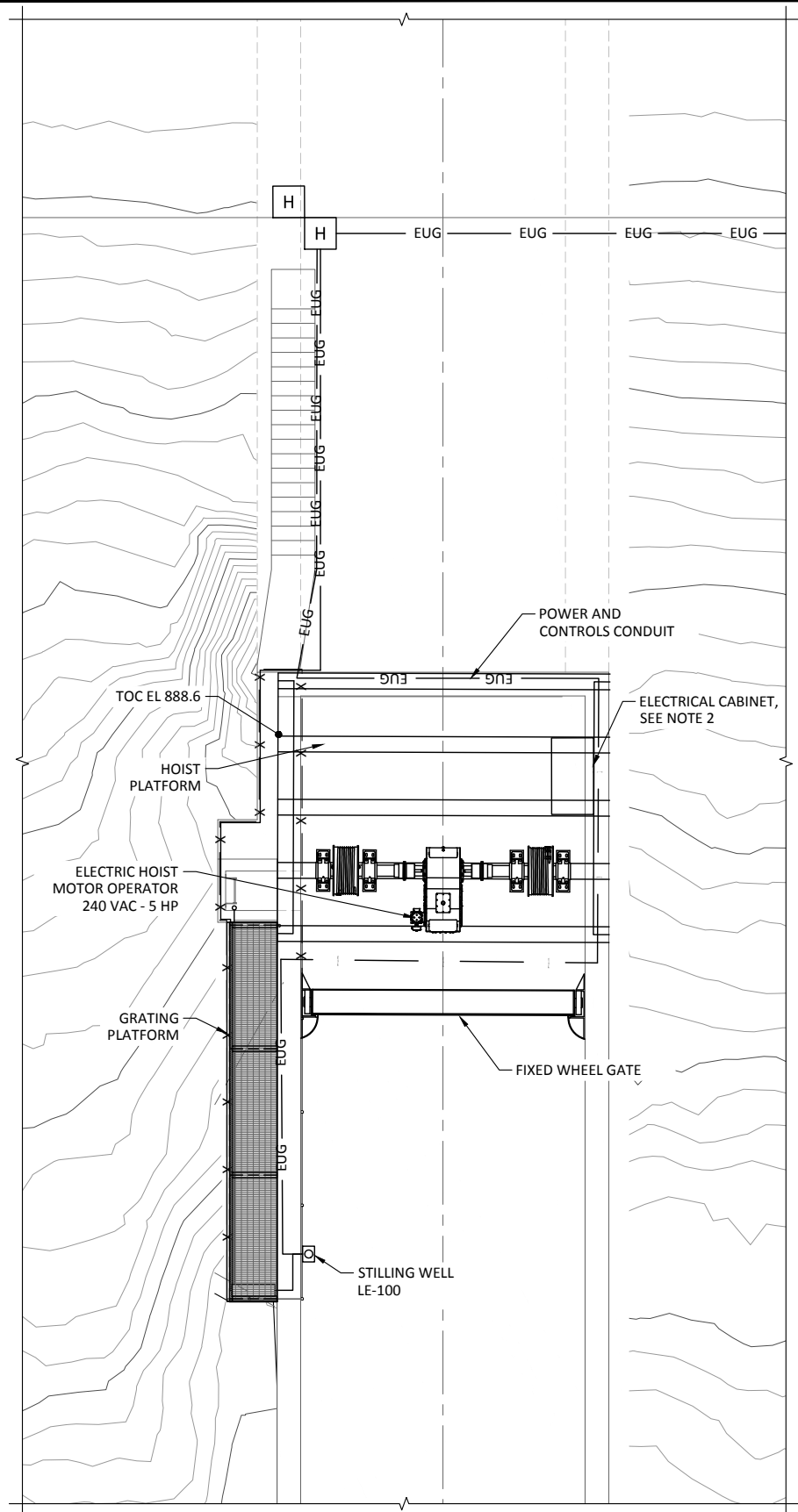
DRAWN J. HOLT

CHECKED J. BAKKEN

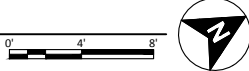
PROJECT DATE 04/26/24

DRAWING

E001



SPILLWAY DETAIL  
SCALE: 3/16"= 1'-0"



- SHEET NOTES:**
- ELEVATIONS SHOWN ARE IN NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88).
  - INSTALL NEW NEMA 250 TYPE 4X SS CABINET ON HANDRAIL OR FRAMING CHANNEL RACK. CABINET SHALL INCLUDE 240VAC SINGLE-PHASE LOADCENTER, FIBER OPTIC CLOSET CONNECTOR HOUSING, AND REMOTE I/O RACK. PROVIDE CABINET COMPLETE AND OPERABLE WITH TERMINAL BLOCKS, SURGE SUPPRESSORS, AND POWER SUPPLIES. INCLUDE LOCKABLE SERVICE ENTRANCE RATED DISCONNECT SWITCH ON EXTERIOR OF CABINET.
  - MOUNT ALL CONDUITS ALONG LOWER PORTION OF HANDRAIL.

PRELIMINARY  
NOT FOR CONSTRUCTION

A	04/26/24	SPE	15% DESIGN	
REV	DATE	BY		DESCRIPTION

WARNING

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**MUNICIPALITY OF ANCHORAGE**

EKLUTNA FISH & WILDLIFE PROJECT
EKLUTNA DAM SPILLWAY MODIFICATIONS
EKLUTNA DAM SPILLWAY ELECTRICAL PLAN, SECTION, AND DETAILS

DESIGNED <u>S. ELLENSON</u>
DRAWN <u>J. HOLT</u>
CHECKED <u>J. BAKKEN</u>
PROJECT DATE <u>04/26/24</u>

DRAWING

**E100**

## **Appendix B. Class 4 Opinion of Probable Construction Costs**

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Phase 2 Engineering  
Eklutna Dam Spillway Modifications

Line Item	Item	Quantity	Unit	Unit Cost	Total Cost	Total
01	<b>DIVISION 01 INDIRECTS</b>					<b>\$460,000</b>
	Mobilization and Establishment of Site Infrastructure	8	%	0.08	\$184,000	
	Contractor General Requirements (Percentage of Direct Cost)	12	%	0.12	\$276,000	
02	<b>SITE CONSTRUCTION AND ACCESS ROADS</b>					<b>\$80,000</b>
	Site Access - Construct Laydown Areas, Turnarounds and Crane Pads	0.5	ACRE	\$160,000	\$80,000	
03	<b>SPILLWAY MODIFICATIONS</b>					<b>\$698,100</b>
	Cofferdam	1	LS	\$30,000	\$30,000	
	Dewatering	3	MO	\$35,000	\$105,000	
	Demo, Misc	1	LS	\$15,000	\$15,000	
	Concrete Demolition, Spillway Concrete	250	CY	\$2,000	\$500,000	
	Hoist Structure and Platforms	16,500	LBS	\$7.50	\$123,800	
	Security Gate Installation	130	LF	\$110	\$14,300	
	Spillway Apron Surface Concrete Repair	40	CY	\$1,500	\$60,000	
04	<b>MECHANICAL EQUIPMENT</b>					<b>\$1,275,000</b>
	Gate Guides and Embeds	1	LS	\$155,000	\$155,000	
	Fixed Wheel Gate Fabrication; 16-ft x 20-ft	25,000	lb	\$20	\$500,000	
	Gate Guides & Fixed Wheel Gate Installation	1	LS	\$90,000	\$90,000	
	Hoist and Gearbox Procurement	1	EA	\$430,000	\$430,000	
	Superstructure, Hoist and Gearbox Installation	1	LS	\$100,000	\$100,000	
05	<b>ELECTRICAL AND TRANSMISSION</b>					<b>\$245,000</b>
	Distribution Panel, Conduit, & Cable	1	LS	\$20,000	\$20,000	
	Instrumentation, Flow Meters, & Gate Controls	1	LS	\$150,000	\$150,000	
	Electrical Install	1	LS	\$40,000	\$40,000	
	Startup & Commissioning	1	LS	\$35,000	\$35,000	
	<b>Project Subtotal (without Division 01)</b>					<b>\$2,298,100</b>
	<b>Project Subtotal</b>					<b>\$2,758,100</b>

**AACE International CLASS 4 Cost Estimate** - Class 4 estimates are generally prepared based on limited information, and subsequently have fairly wide accuracy ranges. Typically, engineering is 1% to 15% complete. They are typically prepared for detailed strategic planning, business case development, projection screening, alternative scheme analysis, confirmation of economic and technical feasibility, selection of a feasible alternative and preliminary budget approval to proceed to the next stage of the project. Virtually all Class 4 estimates use stochastic estimating methods such as cost/capacity curves and factors, historical data and other parametric and modeling techniques. Expected accuracy ranges are from -15% to -30% on the low side and +20% to 50% on the high side, depending on the technological complexity of the project, appropriate reference information, and other risks. Ranges could exceed those shown in unusual circumstances.

Phase 2 Engineering  
Eklutna Dam Spillway Modifications

Project: Eklutna Dam Fixed Wheel Gate  
Location: Anchorage, AK  
Date: 18-Apr-24

**Class 4 Engineer's Estimate -- 15% Design**

<u>Item</u>	<u>Direction Construction Cost</u>	<u>Amount</u>
01	DIVISION 01 INDIRECTS	\$460,000
02	SITE CONSTRUCTION AND ACCESS ROADS	\$80,000
03	SPILLWAY MODIFICATIONS	\$698,100
04	MECHANICAL EQUIPMENT	\$1,275,000
05	ELECTRICAL AND TRANSMISSION	\$245,000
<b>Total Construction Cost</b>		<b>\$2,758,100</b>
 <b><u>Overhead</u></b>		
	GC Overhead and Profit 15.00%	\$414,000
	Construction Bonds 1.25%	\$40,000
<b>Total - Overhead</b>		<b>\$454,000</b>
 <b><u>Direct Cost Contingency</u></b>		
	*Overall Project Contingency: 25.00%	\$803,000
	Escalation: 0.00%	\$0
<b>Total - Contingency</b>		<b>\$803,000</b>
 <b><u>Taxes</u></b>		
	AK Sales Tax 0.00%	\$0
<b>Total - Taxes</b>		<b>\$0</b>
 <b>Median Construction Price - Direct and Indirect</b>		<b>\$4,015,100</b>
<b>Total Construction Price Range (-30% to +50%)</b>		<b>\$2,810,000 to \$6,020,000</b>

Notes:

All costs based on 2024 Construction Dollars

Does not include: interest during construction, legal, financing, or administration costs.

Costs assume a crane can be located on the dam crest for support of work in spillway.

\* Overall Project Contingency is set at 25% due to the current level of project definition and it may be reduced at later stages of design.

## **Appendix C. Structural Calculations**



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## Calculation Cover Sheet



**Project:** Eklutna Fixed Wheel Gate

**Client:** Eklutna Fish & Wildlife **Proj. No.:** 24-021

**Title:** Channel Wall Stability

**Prepared By, Name:** J.Cirafici

**Prepared By, Signature:** \_\_\_\_\_ **Date:** 2/12/2024

**Peer Reviewed By, Name:** G. Clark

**Peer Reviewed, Signature:** \_\_\_\_\_ **Date:** 2/12/2024

**SUBJECT:** Eklutna Fixed Wheel Gate  
 Eklutna Fish & Wildlife  
 Channel Wall Stability

**BY:** J.Cirafici **CHK'D BY:** G. Clark  
**DATE:** 2/12/2024  
**PROJECT NO.:** 24-021

### Purpose

The purpose of this calculation is to determine whether the existing reinforcement in the channel wall is adequate to resist lateral earth pressures after the ogee is removed. The ogee provides lateral bracing of the channel walls along their bottom halves. When the ogee is removed, the walls are no longer laterally braced and serve essentially as cantilevered retaining walls. The following analysis will determine if they can do adequately.

### References

- ACI 318-19: Building Code Requirements for Structural Concrete
- ACI 350-20: Code Requirements for Environmental Engineering Concrete Structures
- ASCE 7-22: Minimum Design Loads for Buildings and Other Structures

### General Information

#### Material Unit Weights

$\gamma_w$ =	62.4 pcf	Water
$\gamma_c$ =	150 pcf	Concrete
$\gamma_s$ =	490 pcf	Steel
$\gamma_{soil}$ =	130 pcf	Soil (Saturated)

### Channel Wall Geometry

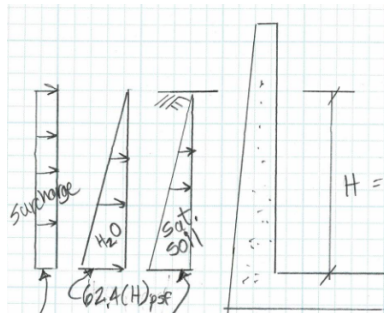
T.O. Wall =	885 ft	Top of Wall Elevation
T.O. Slab =	852 ft	Top of Slab Elevation
B.O. Slab =	849 ft	Bottom of Slab Elevation
T.O. Soil =	879.6667 ft	

$H_{wall}$ =	36 ft	Height of Wall
$H_{soil}$ =	27.667 ft	Height of Retained Soil
$t_{wtop}$ =	18 in	Wall Thickness @ Top
$t_{wbott}$ =	36 in	Wall Thickness @ Bottom

\*Note: Soil Elevation Varies along length of wall. The channel wall is braced laterally by a top slab starting @ Sta. 4+57.33 and by a vertical shaft @ approx. Sta. 4+45.17. The center point between these two bracing points will be considered for design for the closest approximation of pure cantilever action.

### Soil Properties (Assumed)

$\phi$ =	30 deg	Friction Angle
$K_0$ =	0.500	At-Rest Pressure Coeff
$K_a$ =	0.333	Active Pressure Coeff (Rankine)
$K_p$ =	3.000	Passive Pressure Coeff (Rankine)



### Lateral Earth Pressure

\*Wall is assumed to be designed as non-yielding, therefore at-rest pressures will be considered

$\sigma_0 =$	<b>935.133</b> psf	At-rest Lat. Earth Pressure @ Base
$\sigma_w =$	<b>1726.400</b> psf	Lat. Water Pressure @ Base
$\sigma_{sur} =$	<b>100.0</b> psf	Assumed Surcharge Pressure for Construction/Maintenance
LRFD Factor =	<b>1.6</b>	ACI350 9.2

### Wall Strength Check - Cantilever Action

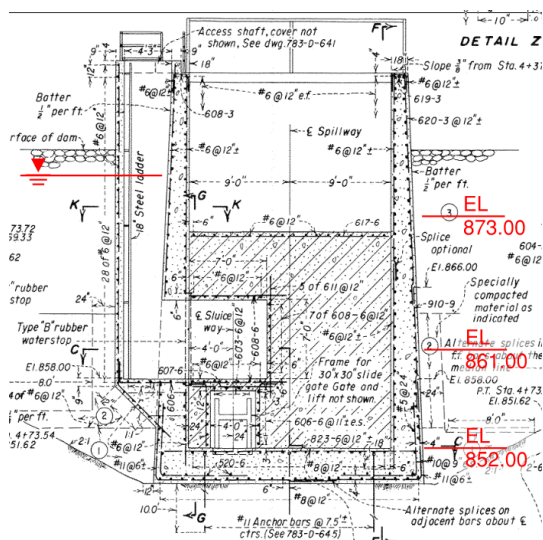
Assumed Concrete Properties:

$f'_c =$	<b>4000</b> psi	Concrete Compressive Strength
$f_y =$	<b>60000</b> psi	Reinforcement Yield Strength

\*Since wall is battered, effective depth of reinforcement. Wall bending capacity will be checked at multiple points along height of wall. The wall will be analyzed in representative 1-ft segment idealized as a singly-reinforced rectangular section.

Cover =	<b>2</b> in	Minimum concrete cover
$\beta_1 =$	<b>0.85</b>	ACI350 Section 10.2.7.3
$\beta =$	<b>EQ 10-6</b>	ACI350 Strain gradient amplification factor per 10.6.4.4
Exposure:	<b>Normal</b>	ACI350 Section 10.6.4.5
Exposure Factor =	<b>320</b>	
Element action:	<b>One-way</b>	
Stress Limit =	<b>20000</b> psi	

### Typical Wall Section:



Wall		South Wall			North Wall		
Elevation		852.00	861.00	873.00	852.00	861.00	873.00
Wall Thickness	in	34.5	30	24	34.5	30	24
bar size		#11	#9	#6	#11	#9	#6
spacing	in	6	6	12	6	6	12
$A_s/ft$	$in^2/ft$	3.12	2.00	0.44	3.12	2.00	0.44
$d_{bar}$	in	1.375	1.125	0.750	1.375	1.125	0.750
$d$	in	31.8125	27.4375	21.625	31.8125	27.4375	21.625
$a$	in	4.59	2.94	0.65	4.59	2.94	0.65
$c$	in	5.40	3.46	0.76	5.40	3.46	0.76
$\beta$		1.10	1.11	1.11	1.10	1.11	1.11
$\epsilon_t$		0.015	0.021	0.082	0.015	0.021	0.082
Check		OK	OK	OK	OK	OK	OK
Min Reinf Ratio		0.005	0.005	0.005	0.005	0.005	0.005
Min Reinf Req'd		1.035	0.900	0.720	1.035	0.900	0.720
Min Steel Check		OK	OK	OK	OK	OK	OK

$$a = \frac{A_s f_y}{0.85 f'_c b}$$

$$c = a / \beta_1$$

\*Shrinkage and Temp steel can be considered to be split between both faces of wall

ACI 350						
$M_{earth}$	kip-ft/ft	119.3	36.6	1.7	119.3	36.6
$M_{hydro}$	kip-ft/ft	220.2	67.6	3.1	220.2	67.6
$M_{surcharge}$	kip-ft/ft	38.27	17.42	2.22	38.27	17.42
$M_{u\_factored}$	kip-ft/ft	604.50	194.73	11.16	604.50	194.73
$M_{u\_service}$	kip-ft/ft	377.81	121.71	6.97	377.81	121.71
Max $f_s$	psi	36,000	36,000	22,261	36,000	36,000
$\gamma$		1.600	1.600	1.600	1.600	1.600
$\phi$ (bending)		0.9	0.9	0.9	0.9	0.9
$S_d$		1.00	1.00	1.52	1.00	1.00
$S_d M_{factor}$	kip-ft/ft	604.50	194.73	16.91	604.50	194.73
$\phi * M_n$	kip-ft/ft	414.44	233.70	42.18	414.44	233.70
Utilization	%	146%	83%	40%	146%	83%
$d - a/2$	in	29.52	25.97	21.30	29.52	25.97
Actual $f_s$	psi	49,228	28,122	8,927	49,228	28,122
s Check ( $f_{sactual} < f_{smax}$ )		No Good	OK	OK	No Good	OK

\*At El. 852.00, there are multiple layers of vertical bars that can be considered to resist bending moments. The capacity is rechecked more thoroughly below:

Outer Layer -->

#11	@	6
$d_{bar}$	=	1.375 in
$A_s/ft$	=	3.12 $in^2/ft$
$d_{outer}$	=	31.8125 in

Inner Layer -->

#10	@	9
$d_{bar}$	=	1.250 in
$A_s/ft$	=	1.69 $in^2/ft$
$d_{inner}$	=	27.8125 in

Combined -->	$A_s/ft$	=	4.81 $in^2/ft$
	$d_{comb}$	=	30.405 in

Bending Strength -->

$a = 7.08$  in  
 $c = 8.33$  in  
 $\beta = 1.19$   
 $\epsilon_t = 0.008$

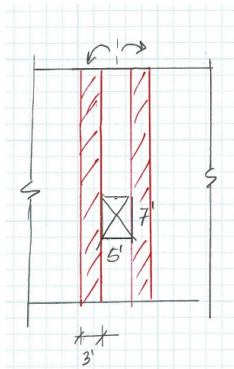
Check **OK**

$\text{Max } f_s = 33,510$  psi  
 $\gamma = 1.600$   
 $\phi$  (bending) **0.9**  
 $S_d = 1.01$   
 $S_d M_{\text{factor}} = 608.84$   
 $\phi * M_n = 581.92$   
 Utilization **105%**  
 $d - a/2 = 31.81$   
 Actual  $f_s = 29,609$

Tensile Reinf. Stress Check **OK**  
 ( $f_{\text{sactual}} < f_{\text{smax}}$ )

Even when considering all layers of reinforcement, the bending moments encountered at the base of the wal still exceed its capacity. As such, struts are recommended at the top of the wall so that the wall functions in simple support bending as opposed to cantilevered action.

\*There is an opening in the south side wall that allows access into the ogee. For the purpose of analysis, load imparted onto the wall above and below the openings are assumed to be resisted by 3-ft segments of the wall directly adjacent to the opening on both sides. A factor of 1.83 has been applied to the loads to account for the 5-ft opening width distributed to the adjacent segments.



$$\text{Ratio} = \frac{3' + 5'/2}{3'} = 1.83'$$

Wall Elevation		South Wall			
		852.00	858.00	861.00	873.00
Wall Thickness	in	34.5	31.5	30	24
bar size		#11	#9	#9	#6
spacing	in	6	6	6	12
$A_s/\text{ft}$	$\text{in}^2/\text{ft}$	3.12	2.00	2.00	0.44
$d_{\text{bar}}$	in	1.375	1.125	1.125	0.750
$d$	in	31.8125	28.9375	27.4375	21.625
$a$	in	4.59	2.94	2.94	0.65
$c$	in	5.40	3.46	3.46	0.76
$\beta$		1.10	1.04	0.86	1.62
$\epsilon_t$		0.015	0.022	0.021	0.082
Check		OK	OK	OK	OK
Min Reinf Ratio		0.005	0.005	0.005	0.005
Min Reinf Req'd		1.035	0.945	0.900	0.720
Min Steel Check		OK	OK	OK	OK

$$a = \frac{A_s f_y}{0.85 f'_c b}$$

$$c = a / \beta_1$$

\*Shrinkage and Temp steel can be considered to be split between both faces of wall

		ACI 350			
$M_{\text{earth}}$	kip-ft/ft	218.3	104.9	67.1	3.1
$M_{\text{hydro}}$	kip-ft/ft	403.0	193.6	123.8	5.6
$M_{\text{surcharge}}$	kip-ft/ft	70.04	42.95	31.88	4.07
$M_{u, \text{factored}}$	kip-ft/ft	1106.24	546.22	356.36	20.42
$M_{u, \text{service}}$	kip-ft/ft	691.40	341.39	222.73	12.76
Max $f_s$	psi	36,000	36,000	36,000	20,000
$\gamma$		1.600	1.600	1.600	1.600
$\phi$ (bending)		0.9	0.9	0.9	0.9
$S_d$		1.00	1.00	1.00	1.69
$S_d M_{\text{factor}}$	kip-ft/ft	1106.24	546.22	356.36	34.45
$\phi * M_n$	kip-ft/ft	414.44	247.20	233.70	42.18
Utilization	%	267%	221%	152%	82%
$d - a/2$	in	29.52	27.47	25.97	21.30
Actual $f_s$	psi	90,087	74,575	51,464	16,337
s Check ( $f_{\text{sactual}} < f_{\text{smax}}$ )		No Good	No Good	No Good	OK

Due to the increased loading from the opening, the segments of the wall adjacent to the opening are inadequate for the applied bending moments.

# Wall Strength Check - Fixed-Pin Condition

\*Recheck wall capacity for condition where lateral bracing is provided @ top of wall. The wall can now be considered fixed-pinned.

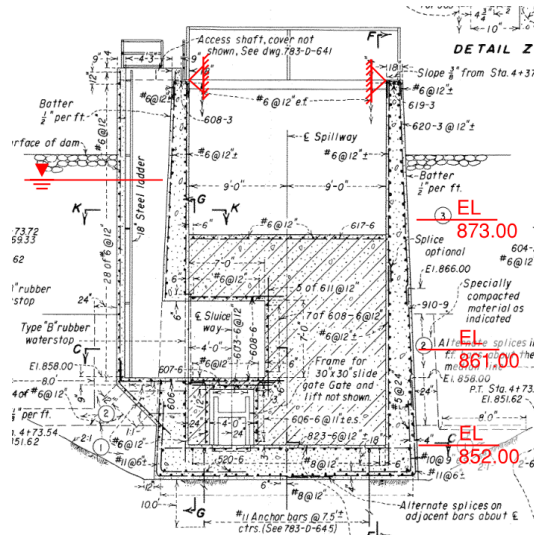
Assumed Concrete Properties:

$f'_c = 4000$  psi      Concrete Compressive Strength  
 $f_y = 60000$  psi      Reinforcement Yield Strength

\*Since wall is battered, effective depth of reinforcement. Wall bending capacity will be checked at multiple points along height of wall. The wall will be analyzed in representative 1-ft segment idealized as a singly-reinforced rectangular section.

Cover = 2 in      Minimum concrete cover  
 $\beta_1 = 0.85$       ACI350 Section 10.2.7.3  
 $\beta = EQ\ 10-6$       ACI350 Strain gradient amplification factor per 10.6.4.4  
Exposure: Normal      ACI350 Section 10.6.4.5  
Exposure Factor = 320  
Element action: One-way  
Stress Limit = 20000 psi

Typical Wall Section:

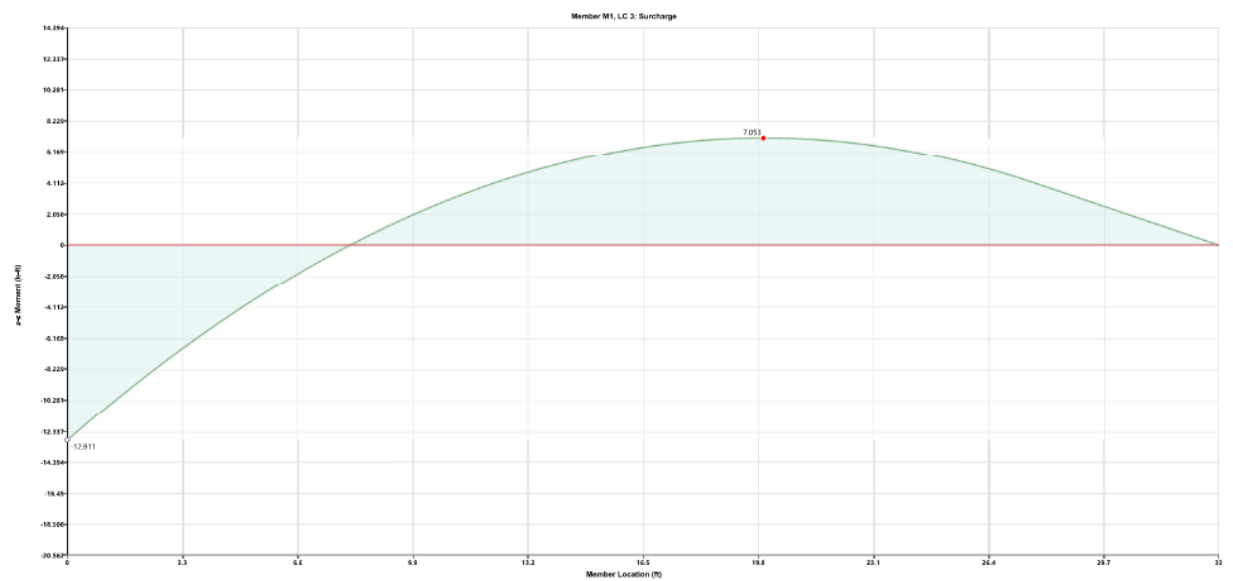
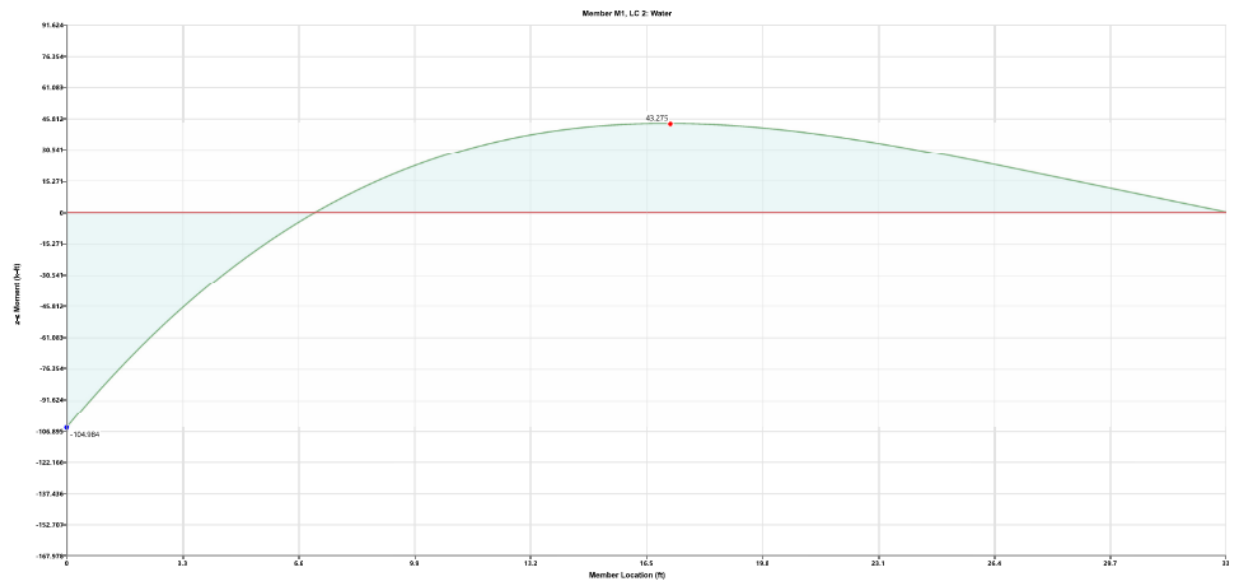
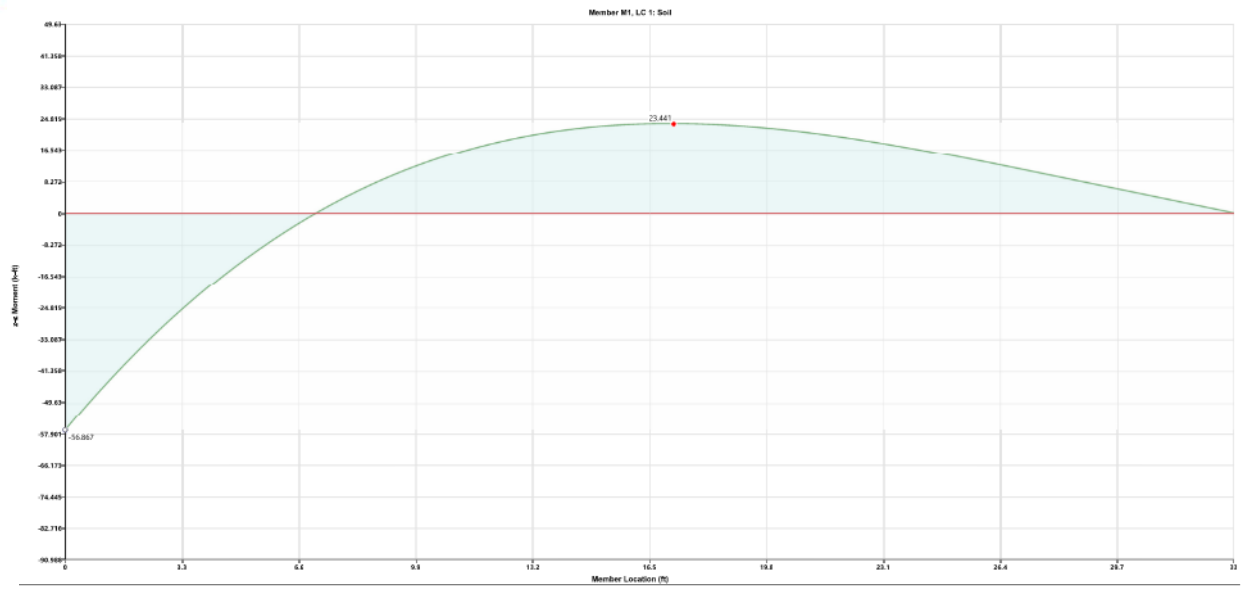


Wall Elevation		South Wall			North Wall		
		852.00	861.00	873.00	852.00	861.00	873.00
Wall Thickness	in	34.5	30	24	34.5	30	24
bar size		#11	#6	#6	#11	#6	#6
spacing	in	6	12	12	6	12	12
$A_s/ft$	in <sup>2</sup> /ft	3.12	0.44	0.44	3.12	0.44	0.44
$d_{bar}$	in	1.375	0.750	0.750	1.375	0.750	0.750
$d$	in	31.8125	27.625	21.625	31.8125	27.625	21.625
$a$	in	4.59	0.65	0.65	4.59	0.65	0.65
$c$	in	5.40	0.76	0.76	5.40	0.76	0.76
$\beta$		1.10	1.09	1.11	1.10	1.09	1.11
$\epsilon_t$		0.015	0.106	0.082	0.015	0.106	0.082
Check		OK	OK	OK	OK	OK	OK
Min Reinf Ratio		0.005	0.005	0.005	0.005	0.005	0.005

$$a = \frac{A_s f_y}{0.85 f'_c b}$$

$$c = a / \beta_1$$



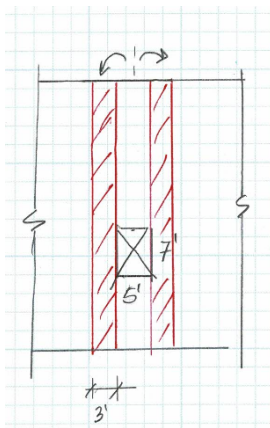


		ACI 350					
M <sub>earth</sub>	kip-ft/ft	56.9	9.9	21.1	56.9	9.9	21.1
M <sub>hydro</sub>	kip-ft/ft	105.0	18.2	38.9	105.0	18.2	38.9
M <sub>surcharge</sub>	kip-ft/ft	12.91	1.33	7.00	12.91	1.33	7.00
M <sub>u_factored</sub>	kip-ft/ft	279.62	47.07	107.18	279.62	47.07	107.18
M <sub>u_service</sub>	kip-ft/ft	174.76	29.42	66.99	174.76	29.42	66.99
Max f <sub>s</sub>	psi	36,000	22,781	22,261	36,000	22,781	22,261
γ		1.600	1.600	1.600	1.600	1.600	1.600
φ (bending)		0.9	0.9	0.9	0.9	0.9	0.9
S <sub>d</sub>		1.00	1.48	1.52	1.00	1.48	1.52
S <sub>d</sub> M <sub>factor</sub>	kip-ft/ft	279.62	69.73	162.50	279.62	69.73	162.50
φ *M <sub>n</sub>	kip-ft/ft	414.44	54.06	42.18	414.44	54.06	42.18
Utilization	%	67%	129%	385%	67%	129%	385%
d - a/2	in	29.52	27.30	21.30	29.52	27.30	21.30
Actual f <sub>s</sub>	psi	22,771	29,385	85,767	22,771	29,385	85,767
s Check (f <sub>sactual</sub> < f <sub>smax</sub> )		OK	No Good	No Good	OK	No Good	No Good

While the base of the wall is now adequate when the wall is considered fixed-pinned, the new moment distribution means the inside face reinforcement is the primary tension reinforcement along the top 2/3 of the wall. This inside face is less heavily reinforced than the outside face, thus limiting the bending capacity.

#### Wall at Chamber Opening:

\*There is an opening in the south side wall that allows access into the ogee. For the purpose of analysis, load imparted onto the wall above and below the openings are assumed to be resisted by 3-ft segments of the wall directly adjacent to the opening on both sides. A factor of 1.83 has been applied to the loads to account for the 5-ft opening width distributed to the adjacent segments.



$$\text{Ratio} = \frac{3' + 5'/2}{3'} = 1.83'$$

Wall		South Wall			
Elevation		852.00	858.00	861.00	873.00
Wall Thickness	in	34.5	31.5	30	24
bar size		#11	#6	#6	#6
spacing	in	6	12	12	12
A <sub>s</sub> /ft	in <sup>2</sup> /ft	3.12	0.44	0.44	0.44
d <sub>bar</sub>	in	1.375	0.750	0.750	0.750
d	in	31.8125	29.125	27.625	21.625
a	in	4.59	0.65	0.65	0.65
c	in	5.40	0.76	0.76	0.76
β		1.10	1.03	0.87	1.62
ε <sub>t</sub>		0.015	0.112	0.106	0.082
Check		OK	OK	OK	OK
Min Reinf Ratio		0.005	0.005	0.005	0.005

$$a = \frac{A_s f_y}{0.85 f'_c b}$$

$$c = a / \beta_1$$

ACI 350					
M <sub>earth</sub>	kip-ft/ft	104.1	13.1	18.1	38.6
M <sub>hydro</sub>	kip-ft/ft	192.1	24.2	33.3	71.2
M <sub>surcharge</sub>	kip-ft/ft	23.6	5.38	2.4	12.8
M <sub>u_factored</sub>	kip-ft/ft	511.70	68.34	86.13	196.14
M <sub>u_service</sub>	kip-ft/ft	319.81	42.71	53.83	122.59
Max f <sub>s</sub>	psi	36,000	24,053	28,663	20,000
γ		1.600	1.600	1.600	1.600
φ (bending)		0.9	0.9	0.9	0.9
S <sub>d</sub>		1.00	1.40	1.18	1.69
S <sub>d</sub> M <sub>factor</sub>	kip-ft/ft	511.70	95.89	101.42	330.99
φ *M <sub>n</sub>	kip-ft/ft	414.44	57.03	54.06	42.18
Utilization	%	123%	168%	188%	785%
d - a/2	in	29.52	28.80	27.30	21.30
Actual f <sub>s</sub>	psi	41,671	40,445	53,775	156,954
s Check (f <sub>sactual</sub> < f <sub>smax</sub> )		No Good	No Good	No Good	No Good

Due to the increased loading from the opening, the segments of the wall adjacent to the opening are inadequate for the applied bending moments.

## **Appendix D. Hydraulic Calculations**

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## Calculation Cover Sheet



**Project:** Eklutna Spillway Modifications

**Client:** Chugach Electric Association

**Proj. No.:** 24-021

**Title:** Hydraulic Calculations

**Prepared By, Name:** Sean Ellenson, P.E.

**Prepared By, Signature:**

**Date:**

**Peer Reviewed By, Name:**

**Peer Reviewed, Signature:**

**Date:**





SUBJECT: Chugach Electric Association  
Eklutna Spillway Modifications  
Hydraulic Calculations

BY: S Ellenson    CHK'D BY: \_\_\_\_\_  
DATE: 4/19/2024  
PROJECT NO.: 21-024

Table of Content

Description	Page
Stage Discharge Relationship	3
<ul style="list-style-type: none"><li>• Calculate the stage discharge relationship for the modified Eklutna Dam Spillway</li></ul>	

SUBJECT: Chugach Electric Association  
Eklutna Spillway Modifications  
Gate Rating Curve

BY: S Ellenson    CHK'D BY: \_\_\_\_\_  
DATE: 4/19/2024  
PROJECT NO.: 21-024

### Purpose

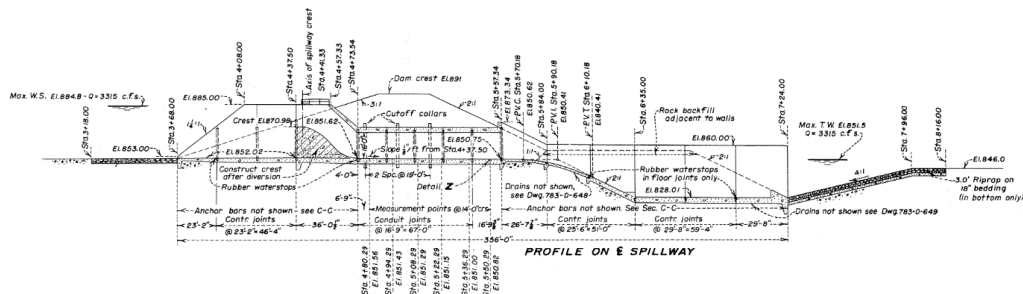
The purpose of this calculation sheet is to develop a preliminary gate rating curve for the proposed fixed wheel gate within the Eklutna Dam Spillway.

### References

- Tullis, J. Paul. (1989). *Hydraulics of Pipelines, Pumps, Valves, Cavitation, Transients*. New York: John Wiley & Sons.
- Miller, D.S. (1990). *Internal Flow Systems, Design and Performance Prediction*. Houston: Gulf Publishing Company.

### Information - Input

Dimensions and elevations of the spillway are obtained from the As-Built Drawings. Eklutna Project - Alaska. Replacement of Eklutna Dam and Spillway. January, 1965.



### Calculation

#### Full Open

Calculations were performed to determine the stage-discharge relationship of the spillway with the existing ogee crest removed and the gate in the fully open position. The spillway was treated as a concrete culvert utilizing open channel flow equations with a tailwater elevation assumed constant at a crest elevation of 855.09 ft msl (NAVD88). The spillway was treated as a concrete culvert utilizing open channel flow equations with a tailwater elevation assumed constant at a crest elevation of 855.09 ft msl (NAVD88).

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.00	0.00	855.6100	0.00	0.0	0-NF	0.00	0.00	0.75	23.49	0.00	0.00
321.50	321.50	859.2700	3.66	0.88	1-S2n	1.35	2.15	1.49	23.49	11.96	0.00
643.00	643.00	861.4200	5.80	2.17	1-S2n	2.14	3.41	2.47	23.49	14.48	0.00
964.50	964.50	863.2200	7.61	3.26	1-S2n	2.80	4.47	3.33	23.49	16.11	0.00
1200.00	1200.00	864.4100	8.80	4.00	1-S2n	3.25	5.17	3.91	23.49	17.05	0.00
1607.50	1607.50	866.3100	10.69	5.19	1-S2n	3.96	6.28	4.86	23.49	18.39	0.00
1929.00	1929.00	867.6900	12.07	6.08	1-S2n	4.50	7.09	5.56	23.49	19.28	0.00
2250.50	2250.50	868.9900	13.38	6.94	1-S2n	5.01	7.86	6.23	23.49	20.06	0.00
2572.00	2572.00	870.2400	14.63	7.78	1-S2n	5.50	8.59	6.88	23.49	20.77	0.00
2893.50	2893.50	871.4300	15.82	8.60	1-S2n	5.99	9.29	7.50	23.49	21.42	0.00
3215.00	3215.00	872.5800	16.96	9.41	1-S2n	6.45	9.97	8.11	23.49	22.02	0.00



# Orifice Control

Calculations were performed to determine the stage discharge relationship at various gate openings with the gate controlling the flow. The flow rate is based on the orifice equation with a discharge coefficient of 0.70.

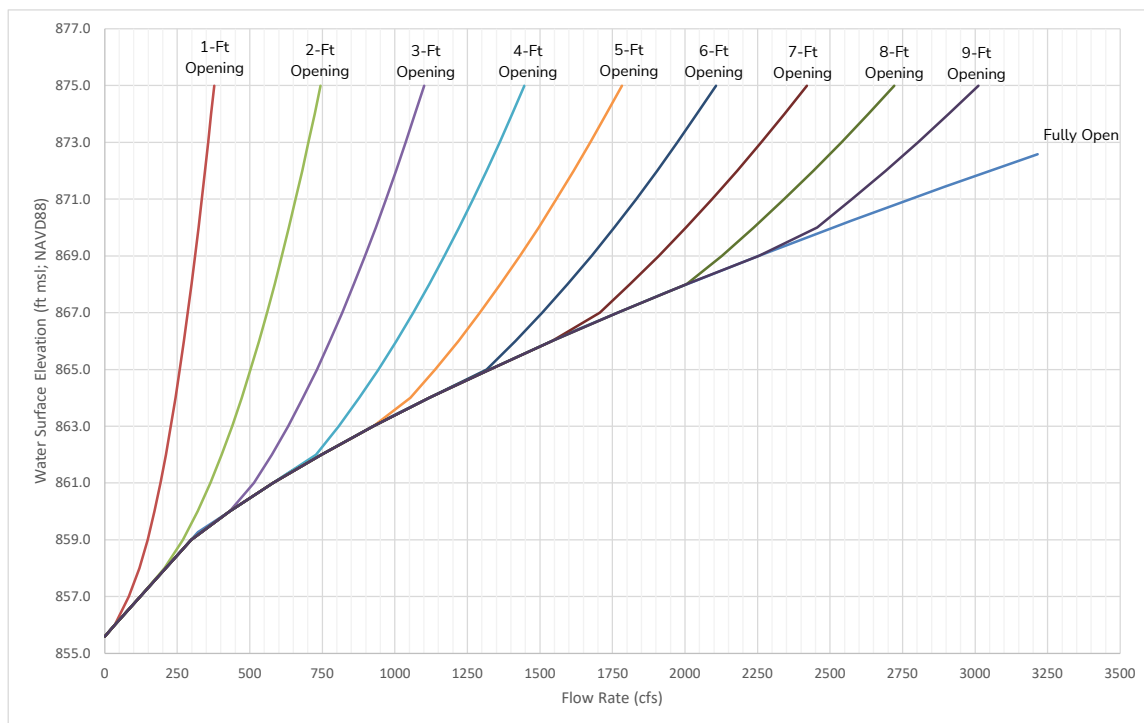
Gate Opening = 1 ft				Gate Opening = 2 ft				Gate Opening = 3 ft			
Opening Elevation = 856.6 ft				Opening Elevation = 857.6 ft				Opening Elevation = 858.6 ft			
Open Area = 18.0 ft <sup>2</sup>				Open Area = 36.0 ft <sup>2</sup>				Open Area = 54.0 ft <sup>2</sup>			
Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)	Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)	Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)
855.6	0.0	Uncontrolled	0.0	855.6	0.0	Uncontrolled	0.0	855.6	0.0	Uncontrolled	0.0
856.0	0.4	Uncontrolled	34.3	856.0	0.4	Uncontrolled	34.3	856.0	0.4	Uncontrolled	34.3
857.0	1.4	Orifice Control	82.5	857.0	1.4	Uncontrolled	122.1	857.0	1.4	Uncontrolled	122.1
858.0	2.4	Orifice Control	119.7	858.0	2.4	Orifice Control	205.6	858.0	2.4	Uncontrolled	209.9
859.0	3.4	Orifice Control	147.8	859.0	3.4	Orifice Control	268.9	859.0	3.4	Orifice Control	297.8
860.0	4.4	Orifice Control	171.3	860.0	4.4	Orifice Control	320.0	860.0	4.4	Orifice Control	430.7
861.0	5.4	Orifice Control	192.0	861.0	5.4	Orifice Control	363.9	861.0	5.4	Orifice Control	513.9
862.0	6.4	Orifice Control	210.6	862.0	6.4	Orifice Control	403.1	862.0	6.4	Orifice Control	576.0
863.0	7.4	Orifice Control	227.8	863.0	7.4	Orifice Control	438.8	863.0	7.4	Orifice Control	631.9
864.0	8.4	Orifice Control	243.7	864.0	8.4	Orifice Control	471.8	864.0	8.4	Orifice Control	683.3
865.0	9.4	Orifice Control	258.7	865.0	9.4	Orifice Control	502.6	865.0	9.4	Orifice Control	731.1
866.0	10.4	Orifice Control	272.8	866.0	10.4	Orifice Control	531.6	866.0	10.4	Orifice Control	776.0
867.0	11.4	Orifice Control	286.2	867.0	11.4	Orifice Control	559.2	867.0	11.4	Orifice Control	818.4
868.0	12.4	Orifice Control	299.1	868.0	12.4	Orifice Control	585.4	868.0	12.4	Orifice Control	858.7
869.0	13.4	Orifice Control	311.4	869.0	13.4	Orifice Control	610.6	869.0	13.4	Orifice Control	897.2
870.0	14.4	Orifice Control	323.2	870.0	14.4	Orifice Control	634.7	870.0	14.4	Orifice Control	934.1
871.0	15.4	Orifice Control	334.6	871.0	15.4	Orifice Control	657.9	871.0	15.4	Orifice Control	969.6
872.0	16.4	Orifice Control	345.7	872.0	16.4	Orifice Control	680.4	872.0	16.4	Orifice Control	1003.9
873.0	17.4	Orifice Control	356.4	873.0	17.4	Orifice Control	702.1	873.0	17.4	Orifice Control	1037.0
874.0	18.4	Orifice Control	366.8	874.0	18.4	Orifice Control	723.2	874.0	18.4	Orifice Control	1069.1
875.0	19.4	Orifice Control	376.9	875.0	19.4	Orifice Control	743.7	875.0	19.4	Orifice Control	1100.3

Gate Opening = 4 ft				Gate Opening = 5 ft				Gate Opening = 6 ft			
Opening Elevation = 859.6 ft				Opening Elevation = 860.6 ft				Opening Elevation = 861.6 ft			
Open Area = 72.0 ft <sup>2</sup>				Open Area = 90.0 ft <sup>2</sup>				Open Area = 108.0 ft <sup>2</sup>			
Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)	Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)	Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)
855.6	0.0	Uncontrolled	0.0	855.6	0.0	Uncontrolled	0.0	855.6	0.0	Uncontrolled	0.0
856.0	0.4	Uncontrolled	34.3	856.0	0.4	Uncontrolled	34.3	856.0	0.4	Uncontrolled	34.3
857.0	1.4	Uncontrolled	122.1	857.0	1.4	Uncontrolled	122.1	857.0	1.4	Uncontrolled	122.1
858.0	2.4	Uncontrolled	209.9	858.0	2.4	Uncontrolled	209.9	858.0	2.4	Uncontrolled	209.9
859.0	3.4	Uncontrolled	297.8	859.0	3.4	Uncontrolled	297.8	859.0	3.4	Uncontrolled	297.8
860.0	4.4	Orifice Control	430.7	860.0	4.4	Uncontrolled	430.7	860.0	4.4	Uncontrolled	430.7
861.0	5.4	Orifice Control	580.2	861.0	5.4	Orifice Control	580.2	861.0	5.4	Uncontrolled	580.2
862.0	6.4	Orifice Control	727.8	862.0	6.4	Orifice Control	746.6	862.0	6.4	Orifice Control	746.6
863.0	7.4	Orifice Control	806.1	863.0	7.4	Orifice Control	925.2	863.0	7.4	Orifice Control	925.2
864.0	8.4	Orifice Control	877.5	864.0	8.4	Orifice Control	1053.2	864.0	8.4	Orifice Control	1118.9
865.0	9.4	Orifice Control	943.5	865.0	9.4	Orifice Control	1138.9	865.0	9.4	Orifice Control	1316.3
866.0	10.4	Orifice Control	1005.2	866.0	10.4	Orifice Control	1218.5	866.0	10.4	Orifice Control	1415.3
867.0	11.4	Orifice Control	1063.3	867.0	11.4	Orifice Control	1293.3	867.0	11.4	Orifice Control	1507.8
868.0	12.4	Orifice Control	1118.4	868.0	12.4	Orifice Control	1364.0	868.0	12.4	Orifice Control	1594.9
869.0	13.4	Orifice Control	1170.9	869.0	13.4	Orifice Control	1431.2	869.0	13.4	Orifice Control	1677.6
870.0	14.4	Orifice Control	1221.1	870.0	14.4	Orifice Control	1495.3	870.0	14.4	Orifice Control	1756.3
871.0	15.4	Orifice Control	1269.4	871.0	15.4	Orifice Control	1556.9	871.0	15.4	Orifice Control	1831.7
872.0	16.4	Orifice Control	1315.9	872.0	16.4	Orifice Control	1616.0	872.0	16.4	Orifice Control	1904.1
873.0	17.4	Orifice Control	1360.8	873.0	17.4	Orifice Control	1673.1	873.0	17.4	Orifice Control	1973.8
874.0	18.4	Orifice Control	1404.2	874.0	18.4	Orifice Control	1728.3	874.0	18.4	Orifice Control	2041.2
875.0	19.4	Orifice Control	1446.4	875.0	19.4	Orifice Control	1781.8	875.0	19.4	Orifice Control	2106.4

Gate Opening = 7 ft				Gate Opening = 8 ft				Gate Opening = 9 ft			
Opening Elevation = 862.6 ft				Opening Elevation = 863.6 ft				Opening Elevation = 864.6 ft			
Open Area = 126.0 ft2				Open Area = 144.0 ft2				Open Area = 162.0 ft2			
Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)	Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)	Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)
855.6	0.0	Uncontrolled	0.0	855.6	0.0	Uncontrolled	0.0	855.6	0.0	Uncontrolled	0.0
856.0	0.4	Uncontrolled	34.3	856.0	0.4	Uncontrolled	34.3	856.0	0.4	Uncontrolled	34.3
857.0	1.4	Uncontrolled	122.1	857.0	1.4	Uncontrolled	122.1	857.0	1.4	Uncontrolled	122.1
858.0	2.4	Uncontrolled	209.9	858.0	2.4	Uncontrolled	209.9	858.0	2.4	Uncontrolled	209.9
859.0	3.4	Uncontrolled	297.8	859.0	3.4	Uncontrolled	297.8	859.0	3.4	Uncontrolled	297.8
860.0	4.4	Uncontrolled	430.7	860.0	4.4	Uncontrolled	430.7	860.0	4.4	Uncontrolled	430.7
861.0	5.4	Uncontrolled	580.2	861.0	5.4	Uncontrolled	580.2	861.0	5.4	Uncontrolled	580.2
862.0	6.4	Uncontrolled	746.6	862.0	6.4	Uncontrolled	746.6	862.0	6.4	Uncontrolled	746.6
863.0	7.4	Orifice Control	925.2	863.0	7.4	Uncontrolled	925.2	863.0	7.4	Uncontrolled	925.2
864.0	8.4	Orifice Control	1118.9	864.0	8.4	Orifice Control	1118.9	864.0	8.4	Uncontrolled	1118.9
865.0	9.4	Orifice Control	1326.5	865.0	9.4	Orifice Control	1326.5	865.0	9.4	Orifice Control	1326.5
866.0	10.4	Orifice Control	1541.0	866.0	10.4	Orifice Control	1541.0	866.0	10.4	Orifice Control	1541.0
867.0	11.4	Orifice Control	1706.0	867.0	11.4	Orifice Control	1768.3	867.0	11.4	Orifice Control	1768.3
868.0	12.4	Orifice Control	1810.6	868.0	12.4	Orifice Control	2005.7	868.0	12.4	Orifice Control	2005.7
869.0	13.4	Orifice Control	1909.6	869.0	13.4	Orifice Control	2126.6	869.0	13.4	Orifice Control	2253.1
870.0	14.4	Orifice Control	2003.6	870.0	14.4	Orifice Control	2236.8	870.0	14.4	Orifice Control	2455.2
871.0	15.4	Orifice Control	2093.5	871.0	15.4	Orifice Control	2341.8	871.0	15.4	Orifice Control	2576.1
872.0	16.4	Orifice Control	2179.6	872.0	16.4	Orifice Control	2442.2	872.0	16.4	Orifice Control	2691.6
873.0	17.4	Orifice Control	2262.5	873.0	17.4	Orifice Control	2538.8	873.0	17.4	Orifice Control	2802.3
874.0	18.4	Orifice Control	2342.4	874.0	18.4	Orifice Control	2631.7	874.0	18.4	Orifice Control	2908.9
875.0	19.4	Orifice Control	2419.7	875.0	19.4	Orifice Control	2721.5	875.0	19.4	Orifice Control	3011.6
Gate Opening = 10 ft				Gate Opening = 11 ft				Gate Opening = 12 ft			
Opening Elevation = 865.6 ft				Opening Elevation = 866.6 ft				Opening Elevation = 867.6 ft			
Open Area = 180.0 ft2				Open Area = 198.0 ft2				Open Area = 216.0 ft2			
Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)	Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)	Headwater Elevation (ft)	Water Depth(ft)	Control	Flow Rate (cfs)
855.6	0.0	Uncontrolled	0.0	855.6	0.0	Uncontrolled	0.0	855.6	0.0	Uncontrolled	0.0
856.0	0.4	Uncontrolled	34.3	856.0	0.4	Uncontrolled	34.3	856.0	0.4	Uncontrolled	34.3
857.0	1.4	Uncontrolled	122.1	857.0	1.4	Uncontrolled	122.1	857.0	1.4	Uncontrolled	122.1
858.0	2.4	Uncontrolled	209.9	858.0	2.4	Uncontrolled	209.9	858.0	2.4	Uncontrolled	209.9
859.0	3.4	Uncontrolled	297.8	859.0	3.4	Uncontrolled	297.8	859.0	3.4	Uncontrolled	297.8
860.0	4.4	Uncontrolled	430.7	860.0	4.4	Uncontrolled	430.7	860.0	4.4	Uncontrolled	430.7
861.0	5.4	Uncontrolled	580.2	861.0	5.4	Uncontrolled	580.2	861.0	5.4	Uncontrolled	580.2
862.0	6.4	Uncontrolled	746.6	862.0	6.4	Uncontrolled	746.6	862.0	6.4	Uncontrolled	746.6
863.0	7.4	D/S Control	925.2	863.0	7.4	Uncontrolled	925.2	863.0	7.4	Uncontrolled	925.2
864.0	8.4	D/S Control	1118.9	864.0	8.4	D/S Control	1118.9	864.0	8.4	Uncontrolled	1118.9
865.0	9.4	D/S Control	1326.5	865.0	9.4	D/S Control	1326.5	865.0	9.4	D/S Control	1326.5
866.0	10.4	D/S Control	1541.0	866.0	10.4	D/S Control	1541.0	866.0	10.4	D/S Control	1541.0
867.0	11.4	D/S Control	1768.3	867.0	11.4	D/S Control	1768.3	867.0	11.4	D/S Control	1768.3
868.0	12.4	D/S Control	2005.7	868.0	12.4	D/S Control	2005.7	868.0	12.4	D/S Control	2005.7
869.0	13.4	D/S Control	2253.1	869.0	13.4	D/S Control	2253.1	869.0	13.4	D/S Control	2253.1
870.0	14.4	D/S Control	2510.3	870.0	14.4	D/S Control	2510.3	870.0	14.4	D/S Control	2510.3
871.0	15.4	D/S Control	2777.3	871.0	15.4	D/S Control	2777.3	871.0	15.4	D/S Control	2777.3
872.0	16.4	D/S Control	2927.2	872.0	16.4	D/S Control	3052.9	872.0	16.4	D/S Control	3052.9
872.6	17.0	D/S Control	3003.2	872.6	17.0	D/S Control	3215.0	872.6	17.0	D/S Control	3215.0

## Results



## **Appendix E. Mechanical Calculations**

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**REVISION:**

**Rev: 0 | Date: April 17, 2024 | Description: Hydrostatic Gate Loading Calcs**

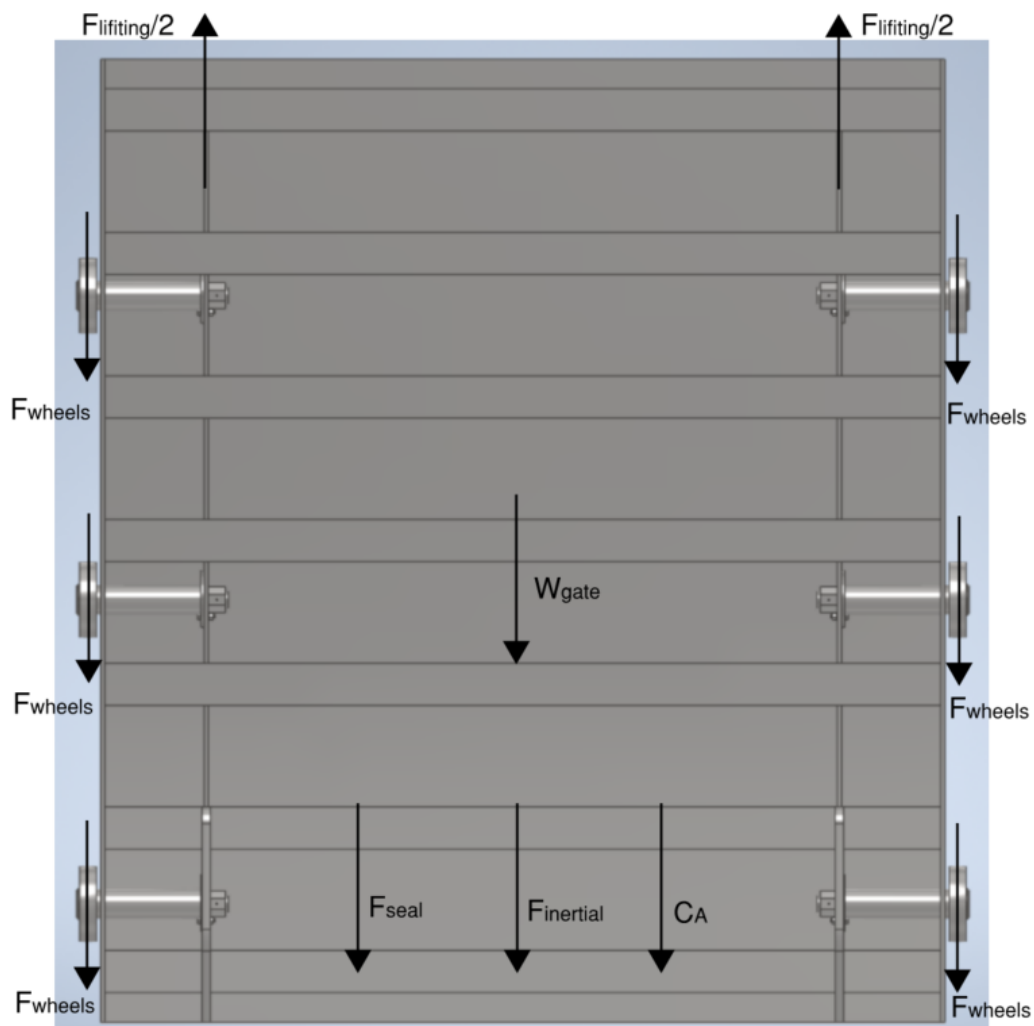
Created By: Anthony DeSantis, EIT

Reviewed By:

**Gate, Hoist Loading**

**A) Discussion**

This calculation sheet will determine the spillway gate loads imparted to the hoist.



### **B) Basic Gate Info**

Gate width, width of gate subject to hydrostatic pressure, skin width per DWG C-004

$$W_{gate} := 16 \text{ ft} + 8 \text{ in}$$

Gate weight with a 10% uncertainty factor

$$P_{gate} := 21000 \text{ lbf} \cdot 1.10 = 23100 \text{ lbf}$$

Ice and mud load

$$C_A := 1.0 (2 \text{ kip}) = 2 \text{ kip}$$

For structural load cases, the atmospheric ice load (C) comes from guidance in USACE EM 1110-2-2107 and ASCE 7. These sources typically find the most extreme loading which does not match the "normal operation" scenario intended for load case A. Because the extreme (max) ice loading is used, the ice and mud load will have an uncertainty factor of 1.0.

### **C) Hydrostatic Loading**

Density of Water

$$\gamma_w := 62.4 \cdot \frac{\text{lbf}}{\text{ft}^3}$$

Sill Elevation

$$EL_{sill} := 855.6 \cdot \text{ft}$$

Top of Skin PL (max normal water level)

$$EL_{pool} := 874.6 \cdot \text{ft}$$

Head on gate

$$h_{eff} := EL_{pool} - EL_{sill} = 19 \text{ ft}$$

Average Effective pressure on Gate

$$p_{eff1} := \gamma_w \cdot \frac{h_{eff}}{2} = 4.117 \text{ psi}$$

Submerged area of gate

$$A_1 := W_{gate} \cdot h_{eff} = 316.667 \text{ ft}^2$$

Force acting on gate

$$F_{hyd} := p_{eff1} \cdot A_1 = 187720 \text{ lbf}$$

### **D) Roller Friction Forces**

Roller Diameter

$$Diam_{roller} := 18 \cdot \text{in}$$

Bearing Bore Diameter

$$Diam_{bearing} := 6.51 \text{ in}$$

Coef of Friction for Self Lube bushings

$$\mu_b := .3$$

Frictional Force@ bushing

$$F_{friction\_axle} := \mu_b \cdot F_{hyd}$$



Frictional Force Seen by gate & hoist at full pool

$$F_{friction\_wheels} := F_{friction\_axle} \cdot \frac{Diam_{bearing}}{Diam_{roller}}$$

$$F_{friction\_wheels} = 20368 \text{ lbf}$$

Effective COF

$$\frac{F_{friction\_wheels}}{F_{hyd}} = 0.109$$

#### **D) Seal Friction**

Pressure at the bottom seal

$$P_{seal\_bottom} := \gamma_w \cdot h_{eff}$$

Coefficient of friction for the side seals (EM 2701, Section 10.2.9.1.1).

$$\mu_{seals} := 0.5$$

Seal dimensions are assumed for now

Side Seal Thickness

$$t_{seal} := \frac{1}{2} \cdot \text{in}$$

Preset Seal Distance (Not available in but this is common value among various projects)

$$\delta_s := \frac{1}{8} \cdot \text{in}$$

Typical J-bulb seal are 60-70 on the Shore A durometer scale

$$H_A := 60$$

Distance from inside of skin plate to centerline of J-bulb side seal

$$D_b := 1.23 \cdot \text{in}$$

#### **E. Calculate Frictional Force From the Bulb Seals**

See LP-212-78 for seal details. The force from side seal friction is defined in EM 2702, 3-4 as:

$$F_{SEALS} = \mu_{SEALS} \cdot S \cdot l + \mu_{SEALS} \cdot \gamma_w \cdot \frac{d}{2} \left( l_1 \cdot \frac{h}{2} + h \cdot l_2 \right)$$

where:

$$S = \frac{3\delta EI}{d^3}$$

Total Length of Both Side Seals:

$$L_{ss} := h_{eff} \cdot 2 = 38 \text{ ft}$$

Width of Side Seal exposed to Hydrostatic Pressure [(width of side seal)-(width of backing bar)] (Width of side seal & backing bar)

$$d_{ss} := 1.91 \cdot \text{in}$$

Modulus of Elasticity for seals. (From Engineering with Rubber, Gent & Campion)

$$E_{seals} := \left( \frac{15.75 + 2.15 \cdot H_A}{100 - H_A} \right) \cdot 1 \cdot \text{MPa}$$

Seal Second Moment of Inertia

$$I_{ss} := \frac{L_{ss} \cdot (t_{seal})^3}{12}$$

Force Induced by Pre-setting Side Seal

$$S_{preset\_ss} := \frac{3 \cdot \langle \delta_s \rangle \cdot E_{seals} \cdot \langle I_{ss} \rangle}{d_{ss}^3} = 134.17 \text{ lbf}$$

Side Seal Friction Force (both sides combined)

$$F_{ss} := \langle \mu_{seals} \cdot \langle S_{preset\_ss} \rangle \rangle + \left( \mu_{seals} \cdot \langle \gamma_w \rangle \cdot 0.5 \cdot \langle d_{ss} \rangle \cdot L_{ss} \cdot (0.5) \cdot \frac{h_{eff}}{2} \right) = 515 \text{ lbf}$$

Conclusion: Assumed bulb seal values show that side seals do not contribute greatly to overall lifting forces.

#### **F. Gate Inertial Loads:**

This section will calculate the inertial loads required to accelerate the gate to full speed.

LCA rope velocity. This values is an estimate. The actual gate speed is calculated at the end of this calculation sheet

$$V_g := 2.0 \frac{\text{ft}}{\text{min}}$$

Time for gate to reach full velocity (estimated)

$$t_g := 0.5 \text{ s}$$

Acceleration of the gate from a stop to full speed.

$$A_g := \frac{V_g}{t_g} = 0.067 \frac{\text{ft}}{\text{s}^2}$$

Inertial force to accelerate the gate.

$$F_i := \frac{P_{gate}}{g} \cdot A_g = 48 \text{ lbf}$$

#### **G. Combined Lifting Forces - Normal Lifting @ Unusual Water Elevation**

$$F_{lifting} := P_{gate} + F_{friction\_wheels} + F_{ss} + C_A + F_i = 46031 \text{ lbf}$$

#### **H. Hoist Load Evaluation:**

This section evaluates the percent that each load contributes to the total hoist load

Percent that each force contributes to the hoist load

<u>Weight of Gate</u>	<u>Ice and Mud:</u>	<u>Seal Friction</u>	<u>Roller Friction</u>	<u>Inertial Load</u>
$P_{gate} = 23.1 \text{ kip}$	$C_A = 2.0 \text{ kip}$	$F_{ss} = 0.52 \text{ kip}$	$F_{friction\_wheels} = 20.37 \text{ kip}$	$F_i = 47.86 \text{ lbf}$
$\frac{P_{gate}}{F_{lifting}} = 50.2\%$	$\frac{C_A}{F_{lifting}} = 4.3\%$	$\frac{F_{ss}}{F_{lifting}} = 1.1\%$	$\frac{F_{friction\_wheels}}{F_{lifting}} = 44.2\%$	$\frac{F_i}{F_{lifting}} = 0.1\%$

Find the difference between gate gravity loads (D) and friction loads ( $F_{ss} + F_{RF}$ ):. To lower under the influence of gravity the gate gravity loads will need to be larger than the friction loads. The ice and mud load will not be included here because for most operations it will not be applied to the gate. A positive value means the gate will lower under it's own weight.

$$P_{gate} - (F_{ss} + F_{friction\_wheels}) = 2.2 \text{ kip}$$

## **REVISION:**

**Rev:** 0 | **Date:** April 17, 2024 | **Description:** Load Case A Calc

Created By: Anthony DeSantis, EIT

Reviewed By:

## **Load Case A (Normal Lifting) Calculation**

### **A) Discussion**

The loads required to lift a spillway gate are a function of the external loads applied to the gate (hydrostatic forces, gravitational forces, friction forces, etc). To calculate the loads at Load Case A, (Normal Operating Condition, NOC, split 50/50 across both hoists) to hoist the Eklutna hoist gate, a free body diagram will be created per Part 1. Operating loads will be applied and the tension in the hoist ropes will be solved. The hoist tension in the wire ropes should experience the largest load at the instant the gate is being lifted from the closed position with a full pool. The assumption with this approach is that a discrete instant of time is utilized in order to account for the inclusion of dynamic forces (side seal and bearing friction). While this is not a true dynamic analysis, performing the calculation at the worse case scenario at its particular instant will yield the most conservative result. This approach has been validated by the inertia calc in Part 1 which indicates that the increase in tension required to bring the gate to full speed is negligible. With the hoist tension known, the torque and speed of the hoisting equipment can be solved for. These torque and speed values will be used to appropriately size equipment for the hoist system.

### **B) Reference Info**

Part 1 - Spillway Gate Loading

### **C) Hoist Configuration**

Lifting Load From Part 1	$F_{lifting} := 46031 \text{ lbf}$	
Drum Diameter (live wrap) & radius	$D_{drum} := 26 \cdot 1.375 \text{ in} = 35.75 \text{ in}$	$r_{drum} := \frac{D_{drum}}{2} = 17.875 \text{ in}$
Reducer Ratios: 250:1 Sumitomo Paramax 9105	$red_{paramax} := 250$	
60:1 BBB4 Gearmotor	$red_{BBB} := 60$	
	$red_{total} := red_{paramax} \cdot red_{BBB} = 15000$	
Rope Diameter	$D_{rope} := 1.375 \cdot \text{in}$	
Number of rope parts	$n_{parts} := 1$	

### **D) Define Efficiency Losses**

Define the mechanical efficiencies of various hoist machinery components

*Hoist drum roller bearing efficiency (EM 1110-2-2610-2.1.d):*

$$\eta_{Hoist\_Bearing} := 0.98$$

*Quadruple reduction parallel shaft reducer efficiency and BBB4 reducer efficiency (see cutsheets):*

$$\eta_{paramax} := 0.90$$

$$\eta_{BBB} := 0.90$$

System Efficiency

$$\eta_{system} := \eta_{paramax} \cdot \eta_{BBB} \cdot \eta_{Hoist\_Bearing}^2 = 0.778$$

### **Wire Rope Sizing: Per EM 1110-2-3200**

Rope Min Breaking Strength for 1-3/8in 6x37 carbon steel wire rope per ASTM-A1023 is 77.7 ton

$$MBS_{Rope\_75in} := 77.7 \text{ tonf}$$

Number of total ropes

$$n_{ropes} := 2$$

EM3200 requires a SF>=5 on rope breaking strength for normal lifting

$$Q_{Rope\_Single\_LCA\_Allow} := 0.2 \cdot (MBS_{Rope\_75in}) = 31080 \text{ lbf}$$

Rope Tension (each side)

$$Q_{Rope\_LCA} := \frac{F_{lifting}}{n_{ropes} \cdot n_{parts}} = 23016 \text{ lbf}$$

Factor of Safety Ropes (over 1 is req'd)

$$FS_{rope\_LCA} := \frac{Q_{Rope\_Single\_LCA\_Allow}}{Q_{Rope\_LCA}} = 1.35$$

### **E) Find speed at different locations**

Motor RPM

$$\omega_{motor} := 1760 \cdot rpm$$

BBB4 Output Shaft Speed

$$\omega_{BBB} := \frac{\omega_{motor}}{red_{BBB}} = 29.333 \text{ rpm}$$

Paramax Output Shaft Speed

$$\omega_{paramax} := \frac{\omega_{BBB}}{red_{paramax}} = 0.117 \text{ rpm}$$

Drum Rotational Speed

$$\omega_{drum} := \omega_{paramax} = 0.117 \text{ rpm}$$

Gate Travel Speed

$$V_{gate} := r_{drum} \cdot \frac{\omega_{drum}}{n_{parts}} = 1.098 \frac{ft}{min}$$

Gate Travel Per Rev

$$D_{rope\_drum} := \frac{\pi \cdot D_{drum}}{n_{parts}} = 9.359 \text{ ft}$$

Gate Travel Per Degree

$$D_{rope\_drum\_deg} := \frac{D_{rope\_drum}}{360} = 0.312 \text{ in}$$

### **F) Find torque at different locations**

Torque on Drum (each side)

$$T_{drum\_LCA} := Q_{Rope\_LCA} \cdot r_{drum} = 34.3 \text{ kip} \cdot ft$$

Torque Output of PARAMAX Red  
9105, 250 ratio

$$T_{paramax.out\_LCA} := \frac{2 \cdot T_{drum\_LCA}}{\eta_{Hoist\_Bearing}} = 71.394 \text{ kip} \cdot ft$$

Torque Input of PARAMAX Red

$$T_{paramax.in\_LCA} := \frac{T_{paramax.out\_LCA}}{red_{paramax} \cdot \eta_{paramax}} = 317.308 \text{ lbf} \cdot ft$$

Torque Output of BBB4 Red 3hp,  
60 ratio

$$T_{BBB\_LCA} := T_{paramax.in\_LCA} = 317.3 \text{ lbf} \cdot ft$$

Torque Output of Motor

$$T_{motor\_LCA} := \frac{T_{BBB\_LCA}}{red_{BBB} \cdot \eta_{BBB}} = 5.876 \text{ ft} \cdot lbf$$

### **G) Find power at different locations**

Motor Power Output

$$P_{motor} := \omega_{motor} \cdot T_{motor\_LCA} = 1.969 \text{ hp}$$

BBB4 Red Power Output

$$P_{cyclo} := \omega_{BBB} \cdot T_{BBB\_LCA} = 1.772 \text{ hp}$$

Paramax Red Power Output

$$P_{paramax} := \omega_{paramax} \cdot T_{paramax.out\_LCA} = 1.595 \text{ hp}$$

Drum Power Output

$$P_{drum} := \omega_{drum} \cdot T_{drum\_LCA} = 0.766 \text{ hp}$$

Select Motor HP:

$$Power_{motor} := 5 \cdot hp$$

### **H) Additional component checks**

Torque Output after first reducer, motor  
rated at 250% of rated motor torque

$$T_{brake} := 2.5 \cdot T_{motor\_LCA} = 14.69 \text{ ft} \cdot lbf$$

Torque requirement of coupling

$$T_{coupling} := \frac{T_{paramax.out\_LCA}}{2} = 428365 \text{ in} \cdot lbf$$

Brake Motor Torque  
(250% rated torque)

$$T_{brake} := 2.5 \cdot \frac{Power_{motor}}{\omega_{motor}} = 37.302 \text{ ft} \cdot lbf$$

### **I) Conclusions**

- 1) Wire rope selection of 1-3/8" has 35% remaining capacity than required for normal operation
- 2) 5hp motor selection is the basis of design
- 3) 60:1 Primary reducer is the basis of design
- 4) 150:1 Secondary reducer is the basis of design



**REVISION:**

Rev: 0 | Date: April 17, 2024 | Description: Load Case B Calc

Created By: Anthony DeSantis, EITReviewed By:**Load Case B (Jammed Gate) Calculation****A) Discussion**

The second load case, Load Case B (LCB), represents a gate jammed situation. LCB is comparable to Case 6 as defined in EM 1110-2-2107 Section 13.5.6 Load Combinations. For this load case, it is assumed that the gate has become jammed during operation and the maximum hoisting loads have been applied to the system.

Per the requirements of USACE EM 1110-2-2610 Section 2.1.b.(1), hoist systems will be designed to not exceed 75% of the yield strength of the components under this loading and 70% of breaking strength for the wire rope per Section 2.2.I. In addition, LCB assumes that the hoist load is imbalanced between the sides of the gate with a 70/30 split. The imbalanced loading assumptions are taken from USACE EM 1110-2-2610 Section 9-2.m.(5) that discusses the 70/30 imbalanced loading assumptions.

Electric motors can typically generate 275% to 325% of rated load torque when experiencing a locked rotor condition. Given the gear reduction typically associated with wire rope hoists, even a small motor can apply very large loads to the gate and supporting structure in a locked rotor condition. The maximum allowable overload applied to the gate will be calculated, and load-limiting devices will be incorporated to de-energize the motor at the corresponding output torque. Load limiting devices would consist of a motor control relay that would measure the loads in the drivetrain, with a 150% LCA motor torque cut-out applying to the LCB design limit.

**B) Reference Info**

Part 1 - Spillway Gate Loading

Part 2 - Hoist LCA

**C) Hoist Torque and Power**

Hoist motor Power

$$Torque_{motor.LCA} := 5.876 \text{ (ft} \cdot \text{lbf)}$$

NEMA B motor locked rotor torque for a 3hp motor as a % of full load torque

[https://www.engineeringtoolbox.com/iec-nema-standards-torques-d\\_741.html](https://www.engineeringtoolbox.com/iec-nema-standards-torques-d_741.html)

Motor torque limited to 150% of LCA motor torque

$$T_{bd} := 150\%$$

Motor RPM

$$\omega_{motor} := 1760 \text{ rpm}$$

Motor Limited Torque

$$T_{motor} := T_{bd} \cdot Torque_{motor.LCA} = 8.814 \text{ ft} \cdot \text{lbf}$$

Drum Pitch Diameter and radius

$$D_{drum} := 35.75 \text{ in} \quad r_{drum} := \frac{D_{drum}}{2} = 17.875 \text{ in}$$

Reducer Ratios: 250:1 Sumitomo  
Paramax 9105

$$red_{paramax} := 250$$

60:1 BBB4 Reducer

$$red_{BBB} := 60$$

BBB4 Red Output Torque (each side)

$$T_{BBB\_LCB} := T_{motor} \cdot red_{BBB} = 0.53 \text{ ft} \cdot \text{kip}$$

Max Reducer Output Torque

PARAMAX 9105 is rated to  $T := 144 \text{ kN} \cdot \text{m}$   
 $T = 106.209 \text{ ft} \cdot \text{kip}$

$$T_{paramax\_LCB} := T_{BBB\_LCB} \cdot red_{paramax} = 132.21 \text{ ft} \cdot \text{kip}$$

Max Rope Force (each side)

$$T_{Rope\_LCB.70} := \frac{70\% \cdot (T_{paramax\_LCB})}{r_{drum}} = 62.13 \text{ kip}$$

$$T_{Rope\_LCB.30} := \frac{30\% \cdot (T_{paramax\_LCB})}{r_{drum}} = 26.63 \text{ kip}$$

Wire Rope Sizing: Per EM 3200

Load Case B allowable tension (EM 1110-2-3200 section 4-5):

Rope Min Breaking Strength for 1-3/8"  
wire rope

$$MBS_{Rope_.75in} := 77.7 \text{ tonf}$$

Number of total ropes

$$n_{ropes} := 2$$

EM3200 requires 70% or less  
for breaking on overload cases

$$T_{Rope\_Single\_LCB\_Allow} := .7 \cdot (MBS_{Rope_.75in}) = 108780 \text{ lbf}$$

Factor of Safety Ropes (over 1 is req'd)

$$FS_{rope\_LCB} := \frac{T_{Rope\_Single\_LCB\_Allow}}{T_{Rope\_LCB.70}} = 1.751$$

Conclusion: 1-3/8" carbon steel IRWC wire rope is adequate for this application.

**REVISION:****Rev:** 2 | **Date:** April 17, 2024 | **Description:** 15% DesignCreated By: Anthony DeSantis, E.I.T.Reviewed By:**Extreme Ice Loading Calculation****A. DISCUSSION:**

This calculation sheet will find the deep bay hoist system loads under extreme ice loading. Ice loading is found using the method in ASCE 7-22

**B. REFERENCES:**

- ASCE - ASCE 7-22 Minimum Design Loads and Associated Criteria for Buildings and Other Structures

**C. EXTREME ICE LOADING:**

1. Height above ground. This is taken as the distance from top of the gate in the closed position to the sill elevation assuming there is no tailrace. The tailrace is assumed to be a factor (ASCE - Section 10.4.3):

$$z := 874.6 \text{ ft} - 855.6 \text{ ft} = 19.00 \text{ ft}$$

2. Height factor (ASCE - Eqn. 10.4-4):

$$f_z := \left( \frac{z}{33 \text{ ft}} \right)^{0.10} = 0.95$$

3. Ice thickness (Figure 1 & ASCE - Figure 10.4-4D):

$$t := 0.75 \text{ in}$$

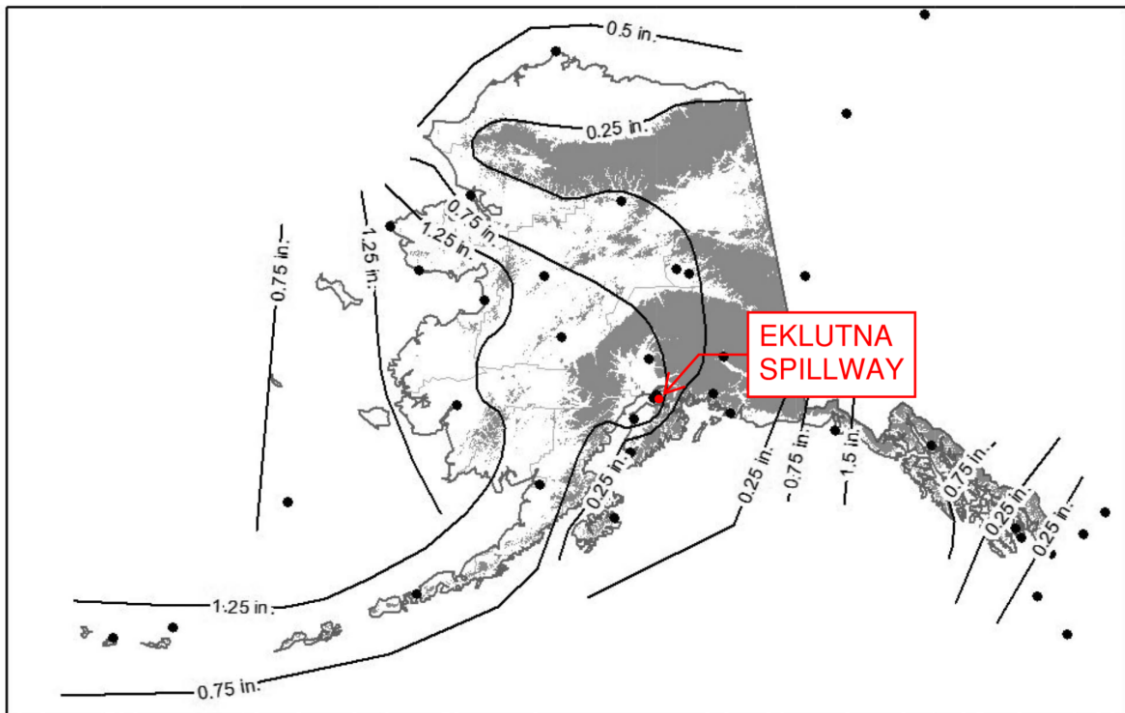


Figure 10.4-3D. Nominal ice thicknesses caused by freezing rain for Alaska for Risk Category IV (1,400-year mean recurrence interval) structures.

Figure 1 - ASCE Ice Thickness for Risk Category IV Structures  
(Figure 10.4-3D)

4. Width of gate (from Inventor model):

$$B := 200 \text{ in}$$

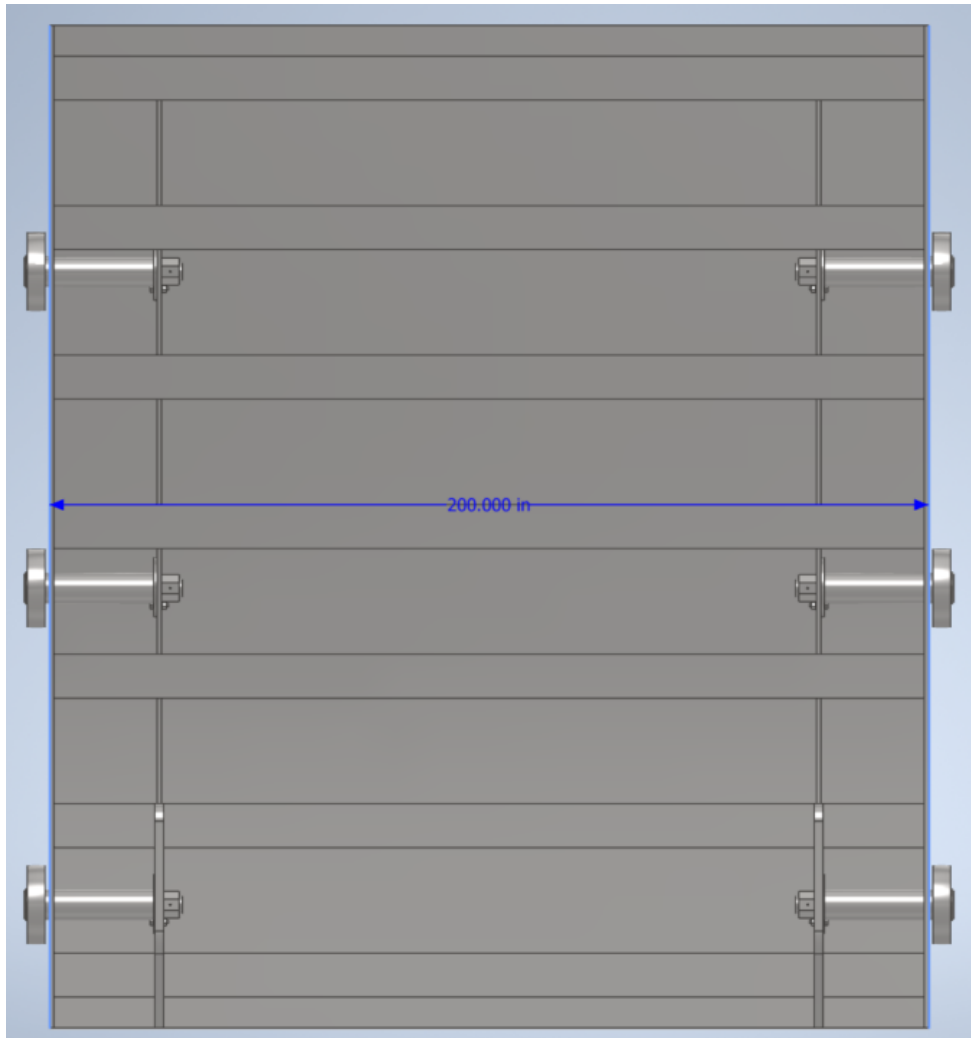


Figure 2 - New Gate Plan and Elevation View Showing Width  
(DWG. S111)

5. Design Ice thickness for freezing rain. Deep bay gates do not meet the dimensional conditions of an escarpment. Topographic factor is taken as 1.0 (ASCE - Section 10.4.4 and 26.8.2):

$$K_{zt} := 1.0$$

6. Design Ice thickness for freezing rain (ASCE - Eqn. 10.4.5):

$$t_d := t \cdot f_z \cdot (K_{zt})^{0.35} = 0.71 \text{ in}$$

7. Deep bay gate characteristic length (Figure 3 & ASCE - Section 10.4-1):

$$D_c := 228.77 \text{ in}$$



Figure 3 - Gate Characteristic Length

8. Volume of freezing rain buildup on deep bay gate:

$$V_I := B \cdot \langle D_c \rangle \cdot t_d = 18.8 \text{ ft}^3$$

9. Ice density (ASCE - Section 10.4.1):

$$\rho_I := 56 \frac{\text{lb}}{\text{ft}^3}$$

10. Extreme ice load:

$$C_A := V_I \cdot \langle \rho_I \rangle = 1.1 \text{ kip}$$